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# Study on the mechanical properties of embankment soil under long-term immersion conditions

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During flood season, embankments are often submerged in high water levels for extended periods, leading to deterioration in their soil mechanics performance and increasing the risk of slope instability and other hazards. In order to investigate the changes in mechanical properties of embankment slopes during long-term water immersion, direct shear tests were conducted. Scanning electron microscopy, chemical composition analysis, and laser particle size analysis were conducted on samples taken at different immersion periods. Clay samples were taken from the embankments at Jiangxinzhou in Nanjing, Jiangsu Province, China. Results showed the shear strength of the soil gradually decreases with the increase of immersion time, while the cohesive force and internal friction angle gradually decrease as well. This suggests that immersion has a softening effect on the shear strength of the soil. As the immersion time increases, the colloidal particles (soluble salt) rapidly dissolves, the microstructure of the soil is destroyed, and sticky particles increases, resulting in a change in the shear strength of the soil. The research results provide a basis for flood control and prevention of embankments immersed in high water levels for long periods during the flood season.

### KEYWORDS

embankment soil, shear strength, long-term immersion, mechanical properties, slope instability

# 1 Introduction

In recent years, due to the frequent occurrence of extreme weather events during the flood season (Xie et al., 2023), the water levels of rivers, lakes, and seas have continued to rise under continuous heavy rainfall and have remained at high levels for a long time. The action of water leads to various forms of deterioration of the embankment soil, which can easily lead to dangerous situations such as landslides, collapses, and pipeline surges (Fujii et al., 2020; Lemmens et al., 2016; Zhou et al., 2023). On the early morning of 8 November 2017, around 5 o'clock, a river collapsed near Zhinan Village, Sanmao Street, Yangzhong City, Jiangsu Province, China. On the morning of 17 August 2020, at 7 o'clock, during an inspection, the government of Luoshui Town, Shifang City, Deyang City, Sichuan Province, China, discovered a dangerous situation in the Zhonghekou section of Shitingjiang, Luoshui Town. Due to the erosion of floodwaters, more than 250 m of the embankment collapsed, resulting in an economic loss of more than 4 million.

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Existing literature has shown that soil strength is a critical parameter in assessing slope stability (Kimura et al., 2014; Lian et al., 2020; Namdar et al., 2020; Sassa et al., 2004; Wen and He, 2012) and that clay exhibits strong sensitivity to water (Philip, 1961). Changes in the water environment can cause changes in soil performance (Liang et al., 2018; Ma et al., 2023; Xu et al., 2012). For instance, Maihemuti et al. (2016) studied the influence of periodic changes in reservoir water level on landslide stability, while Ying et al. (2021) investigated the impact of long-term inundation on the chemical and mechanical properties of silty soil in the Three Gorges Reservoir area. After different immersion periods, Wen and Ji (2018) explored the residual strength changes of different slip zones in five large landslides. During flood season, embankments are often immersed in high water levels for extended periods, leading to the deterioration of soil characteristics due to the interaction between soil and water. It is generally recognized that water level fluctuations play a crucial role in the stability of embankment slopes. However, more attention must be paid to the strength variation and causes of disturbed clay in embankments immersed in high water levels for extended periods during flood season.

In addition, the particle size distribution of soil, soil bonding materials, and mechanical properties are closely related to the strength of the soil (Namdar et al., 2014; Nie et al., 2021; Song and Hong, 2020). The composition and properties of bonding materials determine the strength of the soil. Coarse mineral particles play a skeletal role in the soil structure, while finer particles often fill the gaps between the coarse particles. Bonding materials such as clay minerals and water-soluble salts also fill the gaps between coarse and fine particles, playing a bonding role. Fan et al. (2017) found that the volume and strength of loess samples decrease as the particle size decreases. Manmatharajan et al. (2023) studied how particle size distribution, acceptable powder content, and compressibility affect the strength after liquefaction in simple shear tests. Based on the above literature, laser particle size analyzers can obtain the particle size distribution under different immersion conditions and explore particle composition's influence on the soil's shear strength.

Using scanning electron microscopy, Lin and Cerato (2014) studied the applications of investigating the microstructure of shale weathering expansive soil during the expansion-contraction cycle, while Zhu et al. (2022) observed changes in the loess's particle and pore size distribution. These studies demonstrate the potential of scanning electron microscopy for analyzing changes in soil structure and pore characteristics (Li et al., 2019; Sun et al., 2023; Tang et al., 2020). The impact of immersion time on disturbance-induced changes in clay structure in embankments remains unclear.

The embankment project refers to the water-retaining buildings built along rivers, lakes, canals, coasts, or flood zones. Flood zones, the edge of the part, are essential to China's flood control engineering system. In recent years, the water level of rivers, lakes, and the sea has continued to increase due to the frequent occurrence of extreme weather during the flood season. The embankment has been in a submerged environment for a long time, resulting in an embankment deterioration phenomenon that can easily lead to landslides, subsidence, tube surge, and other dangerous situations. The research on the performance change of embankment body soil under long-term submergence conditions is still relatively small. This paper studies the deterioration mechanism of embankment flooding under long-term high-water level conditions during flood season.

Based on the above considerations, this research used direct shear tests, X-ray fluorescence spectrometry, laser particle size analyzers, and scanning electron microscopy techniques to explore the reasons for the change in the strength of disturbed clay in embankments with immersion time. The gray correlation entropy analysis algorithm was used to analyze the correlation between particle composition and soil strength indicators. The research results provide a basis for flood control and prevention of embankments immersed in high water levels for long periods during the flood season.

# 2 Materials and methods

### 2.1 Materials

Figure 1 presents a schematic diagram of the sampling positions. Per the soil mechanics testing standard (GB/T50123-2019) (2019), the specific gravity, liquid limit, maximum dry density, and optimum moisture content of the soil were determined through experimentation and shown in Table 1.

The soil was placed in a drying box at 105°C to dry and then crushed. The dried soil sample was passed through a 2 mm sieve. It was then sprayed with distilled water until the water content reached 10%, stirred well, and sealed in a plastic bag for 48 h to allow the sample to be water-balanced.

# 2.2 Methods

Weigh according to the target dry density  $(1.5 \text{ g/cm}^3)$ . Please place them in a mold with a diameter of 61.8 mm and a height of 20 mm, and samples are then made using the press sample method. After compaction, remove the molded samples. Eight groups of four samples each were prepared as remodeled soil samples with 10% water content and 1.5 g/cm<sup>3</sup> density. After saturation by vacuum pumping method, one group was directly taken out and immersed in water for 1, 3, 5, 7, 10, 15, and 20 days.

Samples with different immersion times were placed in a shear box. The upper box was fixed, while the lower box could slide horizontally. Vertical pressures of 100, 200, 300, and 400 kPa were applied, respectively. Then, the horizontal shear force was gradually applied to the lower box of the shear box until the sample was sheared. The shear rate was set at v = 2 mm/min, the shear displacement was 6 mm, and the readings were recorded for every 0.2 mm shear displacement.

After drying and cooling the samples with different immersion times, they were ground into fine particles using a grinding rod. The prepared samples were stored in preservation bags. The samples were immersed in water for 0 and 20 days, respectively. The soil was tested using an X-ray fluorescence spectrometer.

The gradient of sample immersion time was 0, 1, 3, 5, 7, 10, 15, and 20 days. Appropriate quantities of samples were taken at each



#### TABLE 1 Basic physical characteristics.

Gs	Maximum dry unit weight	Optimum water content (%)	w <sub>L</sub> (%)	w <sub>p</sub> (%)	I <sub>p</sub>
2.72	1.68 g/cm3	19.86	39.17	19.48	19.69

immersion time point for the tests. Perform particle size analysis on soil samples using a laser particle size analyzer. The impact of particle composition on soil shear strength was explored using the gray correlation entropy analysis algorithm.

The gradient of water immersion time was 0, 5, 10, and 20 days in order, and the samples were dried. A cubic rod of approximately  $1 \text{ cm} \times 1 \text{ cm} \times 2 \text{ cm}$  was cut from the middle of the ring knife sample after different immersion times and used as a scanning electron microscope sample. The specific operation steps of SEM testing were mentioned by Ni et al. (2020).

# 3 Results and analysis

### 3.1 Analysis of direct shear test results

The shear stress-shear displacement curves of samples at different immersion times were obtained through direct shear tests, as shown in Figure 2. It can be observed that both vertical stress and immersion time have significant influences on the shear stress-shear displacement curves of the soil.

Based on the experimental results, the failure strength curve of the sample under different immersion days and the shear strength curve of the test under different vertical pressures were drawn. As shown in Figure 3, the shear strength of the sample decreases with the increase of immersion days, indicating that immersion has a softening effect on the shear strength of the soil. At 400 kPa, with the increase of immersion days, the cumulative attenuation value of the failure and shear strengths is 18.4 kPa. When the vertical stress is low, the attenuation of shear strength is also low. When the axial pressure is 100 Pa, the cumulative attenuation value is 10.7 kPa.

The relationship curve between cohesive force, internal friction angle, and immersion time is shown in Figure 4. The cohesive force gradually decreases with the increase of immersion time, while the internal friction angle also gradually decreases. After immersion for 20 days, the cohesive force of the clay decreased by 57.6%, while the internal friction angle decreased by 8.43%, with a smaller decrease range.

# 3.2 Analysis of change in chemical composition

 $SiO_2$ ,  $AL_2O_3$ , and  $Fe_2O_3$  are the soil's primary chemical binding materials (Liu zhikui et al., 2017; Mou Chunmei and Wei Yux, 2019), with a total content of 88.01% of the soil's chemical composition.

As shown in Figure 5, after immersion for 20 days, the SiO<sub>2</sub> content increased by 1.69%, related to the hydrolysis equilibrium of SiO<sub>2</sub> in the aqueous solution. AL<sub>2</sub>O<sub>3</sub> and Fe<sub>2</sub>O<sub>3</sub> content was lower than those of the unsoaked samples. The AL<sub>2</sub>O<sub>3</sub> and Fe<sub>2</sub>O<sub>3</sub> in the soil react with the aqueous solution, destroying the original binding structure of the soil and further crushing the original soil structure. The Na<sub>2</sub>O content decreased mainly due to the alternation adsorption of cations, which caused Na<sup>+</sup> to leave the soil with the infiltrating liquid. CaO mainly exists in carbonate and sulfate minerals, and MgO and K<sub>2</sub>O mainly exist in calcite and orthoclase, respectively. The decrease in their content reflects the dissolution of corresponding mineral components. All these chemical reactions are beneficial to weaken the structural strength of the soil.



# 3.3 Analysis of LPS test

Using a laser particle size analyzer, the experimental particle size distribution was tested, and the sand content (d > 0.075 mm), powder content (0.005 mm < d < 0.075 mm), and clay content (d < 0.005 mm)

of the soil samples after different immersion times were statistically analyzed and plotted. As shown in Table 2, the particle size of the soil continuously became finer with the increase in immersion time. For example, after immersion for 20 days, the mass fraction of sand content (d > 0.075 mm) decreased by 9.47% due to its smaller base but significant



FIGURE 3

Failure strength curve and shear strength line of samples under different immersion days. (A) Relationship between sample shear stress and immersion time; (B) Relationship between sample shear strength and vertical stress.



decomposition, while the mass fraction of clay (d < 0.005 mm) increased by 5.83%, and the mass fraction of powder (0.005 mm < d < 0.075 mm) increased by 3.64%. Throughout the immersion process, the changes in soil particles mainly involved the transformation of medium and fine sand into powder, while powder gradually transformed into clay, and the mass fraction of sand gradually decreased. In contrast, the mass fraction of clay and powder gradually increased. Therefore, it can be concluded that once the soil is immersed in water, its structure is destroyed, the coarse particles disperse and decompose, and the content of medium and fine particles increases. With the increase of immersion days, the coarse particles further disperse and decompose, and the content of clay and powder gradually increases.

After different immersion periods, statistical analysis was conducted on the usual particle size (including the characteristic particle size with cumulative particle distribution of 10%, 30%, 60%, and 99%). The specific data and changes are shown in Table 3. It can be observed from Table 3 that with the increase in immersion time, the characteristic particle size with cumulative particle distribution of 10%, 30%, 60%, and 99% of the soil particles continuously decreased. The change of the characteristic particle size with a cumulative particle distribution of 10% and 30% slowly decreased with the increase of immersion days. The decrease of the characteristic particle size with a cumulative particle distribution of 60% was divided into the pre-immersion period (0-10 days) and the preand post-immersion period (10-20 days). The usual particle size with a cumulative particle distribution of 99% decreased from 0 to 1 day.

The correlation between the content of clay particles, fine particles, sand particles, and soil strength indicators was calculated using gray correlation entropy analysis. The grey correlation analysis method was used to determine various factors' varying degrees of influence on the dependent variable in complex systems (Deng et al., 2023; Zhang et al., 2019). Zhang Qishan et al. (1996) analyzed the two significant shortcomings of the existing gray correlation methods in 1996 and, based on Deng Julong's gray correlation theory, introduced the concept of gray entropy and proposed the gray correlation analysis method. The specific introduction of the gray correlation entropy analysis method is as follows (Wang et al., 2014).

If a gray correlation factor set denoted as X contains a reference sequence (dependent variable)  $X_0$  and comparison sequences (independent variables)  $X_j$  ( $j = 1, 2, \dots, k; i = 1, 2, \dots, m$ ), then X can be represented as:



TABLE 2 Proportion of clay, silt and sand content (%).

	0 d	1 d	3 d	5 d	7 d	10 d	15 d	20 d
0-5um	28.83	29.3	30.61	30.98	31.79	32.87	34.66	34.66
5-75um	57.92	57.62	59.19	59.93	60.37	60.90	61.11	61.56
>75um	13.25	13.08	10.2	9.09	7.84	6.23	4.23	3.78

TABLE 3 Characteristic particle size.

	0 d	1 d	3 d	5 d	7 d	10 d	15 d	20 d
<i>d</i> <sub>10</sub> (mm)	1.479	1.408	1.379	1.392	1.344	1.303	1.149	1.189
<i>d</i> <sub>30</sub> (mm)	5.462	5.252	4.732	4.652	4.302	4.263	3.862	3.608
<i>d</i> <sub>60</sub> (mm)	11.12	10.83	10.57	10.44	10.11	9.713	9.632	9.578
<i>d</i> <sub>99</sub> (mm)	130.7	115.8	113.3	111.5	106.8	98.13	96.14	87.37

$$X = \begin{cases} X_0 = (x_0(1), x_0(2), \dots, x_0(i), \dots, x_0(m)) \\ X_1 = (x_1(1), x_1(2), \dots, x_1(i), \dots, x_1(m)) \\ \dots \\ X_j = (x_j(1), x_j(2), \dots, x_j(i), \dots, x_j(m)) \\ X_k = (x_k(1), x_k(2), \dots, x_k(i), \dots, x_k(m)) \end{cases}$$

To normalize each sequence of X, there are two common methods. If the first data point in a sequence is not equal to zero, then each data point in the sequence is typically divided by the first data point. If the first data point is equal to zero, then the average of all data points in the sequence can be used as the divisor. Normalizing the sequences using these methods results in factor set X'.

$$X' = \begin{cases} X'_{0} = (x'_{0}(1), x'_{0}(2), \cdots, x'_{0}(i), \cdots x'_{0}(m)) \\ X'_{1} = (x'_{1}(1), x'_{1}(2), \cdots, x'_{1}(i), \cdots x'_{1}(m)) \\ \cdots \\ X'_{j} = (x'_{j}(1), x'_{j}(2), \cdots, x'_{j}(i), \cdots x'_{j}(m)) \\ X'_{k} = (x'_{k}(1), x'_{k}(2), \cdots, x'_{k}(i), \cdots x'_{k}(m)) \end{cases}$$

Using the comparative sequence as a reference, the gray correlation coefficient of each comparative sequence is calculated using the following formula:

$$\gamma(x_{0}(i), x_{j}(i)) = \frac{\min_{j} \min_{i} |x'_{0}(i) - x'_{j}(i)| + \xi \max_{j} \max_{i} |x'_{0}(i) - x'_{j}(i)|}{|x'_{0}(i) - x'_{j}(i)| + \xi \max_{j} \max_{i} |x'_{0}(i) - x'_{j}(i)|}$$

According to the mapping relationship between the distribution of gray correlation coefficients between the reference sequence and the comparison sequence, the mapping value  $P_h$  ( $h = 1, 2, \dots, m$ ) is obtained, where the coefficient  $\xi \in [0,1]$  is the resolution coefficient and is usually set to 0.5.

$$P_{h} = \frac{\gamma(x_{0}(i), x_{j}(i))}{\sum_{i=1}^{m} \gamma(x_{0}(i), x_{j}(i))}$$

 $P_h$  also known as the density value of a distribution. For gray connotation sequence  $A = (a_1, a_2, \dots, a_r, \dots)$  (each element is not less than 0, and the sum is equal to 1), its gray entropy is:

$$H = -\sum a_r \ln a_r$$

By substituting  $P_h$  into Formula, we obtain the grey correlation entropy of each comparison sequence.

$$H_{j} = -\sum P_{h} \ln P_{h}$$

Finally, the grey entropy correlation degree of each comparative sequence was obtained through the following formulaic calculation:

$$E_{r}(X_{j}) = \frac{H_{j}}{lnm}$$

Sequence  $X_a$  is more closely aligned with the reference sequence than sequence  $X_b$ , and as a result, it makes a greater contribution to the alignment of sequence  $X_0$ .

As shown in Figure 6, the results indicate the highest gray correlation degree between cohesion and sand content, followed by silt and clay particles. The gray correlation degree of the three is more significant than 0.95, all of which have vital statistical significance. The gray correlation degree between the internal friction angle and the silt content is the highest, followed by clay particles, and the sand content is the lowest. The gray correlation degree of the three is more significant than 0.95, all of which have vital statistical significance.



FIGURE 6

Grey entropy relevance between clay particles, powder particles, sand particles and a indicators of strength. (A) Grey entropy relevance between clay particles, powder particles, sand particles and cohesion; (B) Grey entropy relevance between clay particles, powder particles, sand particles and internal friction angle.



### FIGURE 7

SEM images of samples under different immersion days. (A) 0 d; (B) 5 d; (C) 10 d; (D) 20 d.



## 3.4 Analysis of SEM test

As depicted in Figure 7, SEM microstructure images of samples immersed for 0, 5, 10, and 20 days are presented, the magnification factor being  $\times 2000$ . With the increase in immersion time, the soil particles were still composed of cohesive or aggregated flakes, forming a stacked structure. However, the original compactness of the soil weakened, and the orientation and arrangement could be more explicit. The particles within the field of view were severely fragmented, the area decreased, and the boundary between particles and pores blurred. The stacking of particles and pores increased, and there were more small voids at the edges of particle-to-particle or particle-to-edge contact, with some local inter-particle pores connected. Overall, the soil structure within the field of view became loose as the immersion time increased.

There was a general decreasing trend in pore area and an increasing trend in pore number. This indicates that as the number of days of immersion increased, the colloidal particles (soluble salts) in the soil dissolved rapidly, large pores decreased, and tiny pores increased. The overall soil structure within the field of view becomes loose as the immersion days increased.

# 4 Discussions

Based on the study's results, the process of soil property change under long-term immersion was sorted out, as shown in Figure 8. The dissolution of a small amount of colloidal particles in the soil after long-term immersion caused a decrease in the force between soil particles. The dispersion of soil particle aggregates led to particle size refinement, and the cohesive force was also reduced. The content of fines and cohesive particles in the soil increased after particle size refinement and the increase in the fines content led to a decrease in the angle of internal friction.

Under long-term immersion, a series of complex microphysical and chemical reactions occur, which cause changes in the particle size of the soil. With the increase of immersion time, and the number of fine particles increases rapidly. When the content of clay in the soil increases, although increasing the content of fine particles can help to produce more particles with a "bonding" effect, the soil skeleton is destroyed due to the sharp decrease of coarse particle content. The interaction between coarse and fine particles weakens, resulting in a decreasing trend of soil cohesion.

During the soil samples' shear, the friction between irregular and coarse particles on the surface primarily provides the frictional resistance between soil particles. Fine particles exist between coarse particles in the soil sample, so the more fine particles present in the soil sample, the more these particles will soften under the surrounding moisture during shear failure and act as a "lubricant" on the surface of the coarse particles, reducing the frictional resistance between them. As a result, the internal friction angle of the soil sample decreases.

# **5** Conclusion

In the embankment remodeling soil study, the samplings were subjected to straight shear tests, chemical composition tests, laser particle size tests, and SEM tests at different immersion times. The following conclusions can be drawn based on the experimental results and the previous discussion:

- 1. Conducting direct shear tests on soil samples with different immersion times, it was discovered that the influence of immersion time on the shear strength of remolded clay is exceptionally significant. This was mainly manifested by a continuous weakening of the shear strength as the immersion time increased. The cohesion and the angle of internal friction decreased gradually with the increase of immersion time, but the change in the internal friction angle was insignificant.
- 2. The long-term physicochemical effects of water on soil are significant: laser particle size analyzer tests show that long-term immersion leads to the refinement of soil particle size, with a consequent increase in the number of fine particles. X-ray fluorescence spectrometry tests show changes in the chemical composition of the specimens after 20 days of immersion, and these chemical reactions favor the weakening of the structural strength of the soil and the promotion of pore development. The refinement of soil particles was also associated with changes in chemical composition. After processing the SEM images of the samples that had been soaked for different periods, it was observed that continuous immersion increased the number of

pores in the samples, the number of large pores decreased, the number of tiny pores increased, and the structure of the soil in the field of view became looser.

3. Under long-term immersion, the soil undergoes a series of complex microphysical and chemical reactions, resulting in the macroscopic manifestation of strength weakening and water immersion aging. The mechanism analysis is as follows: in the early stage of immersion, the colloidal particles (easily soluble salt) dissolved rapidly, the soil microstructure was destroyed, and the cohesive force significantly decreased; with the increase of immersion time, the content of clay particles increased, and the fine particles and clay particles content of the soil increased after particle size refinement, which led to a decrease in the internal friction angle.

# Data availability statement

The raw data supporting the conclusion of this article will be made available by the authors, without undue reservation.

## Author contributions

KZ: Conceptualization, Data curation, Investigation, Methodology, Writing-original draft, Writing-review and editing. DF: Conceptualization, Funding acquisition, Investigation, Writing-review and editing. ZW: Formal Analysis, Writing-original draft, Writing-review and editing.

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# Conflict of interest

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