



Tensile Performance of High Ductility Cementitious Composites With Recycled Powder From C&D Waste

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The effects of recycled powder (RP) on hydration, micro-structures, compressive strength and tensile properties of high-ductility cementitious composites (HDCC) were studied. Three mass ratios of RP ranging from 10, 30 to 60% were employed to partially replace the cement. The hydration kinetics of RP-amended matrix was measured to investigate the influence of RP on the cementitious hydration process and the phase development was quantified by the thermal gravimetric analysis. Mechanical properties, including the compressive and tensile properties of HDCCs were obtained at 28 days. The morphology of reinforced polyethylene fiber at the fracture surface was obtained by the environmental scanning electron microscope (SEM) analysis.

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INTRODUCTION

Recycled powder (RP) with the particle size smaller than 150 μ m is a byproduct during the concrete (clay brick) crushing process. It is estimated that RP accounts for roughly 20% of the entire concrete and clay brick waste (Sun et al., 2017). These tiny grains should be properly utilized to avoid the potential pollution to the air and underground water (Xiao et al., 2016).

Over the past decades, quite a few researches have been conducted to investigate the effects of alternative cementitious materials to the mechanical properties of concrete (Li et al., 2007, 2009; Yang et al., 2007; Zhang and Zhang, 2018; Wang et al., 2019). In recent years, RP was utilized to substitute a fraction of cement in the mixture as a supplementary cementitious material like fly ash and metakaolin. It was revealed that the RP would speed up hydration at the early age while reducing total hydration heat release (Liu et al., 2014). On the basis of the micro-mechanics designing theory, the coarse aggregate in the high-ductility cementitious composites (HDCC) mixture should be removed and the sand/binder ratio was largely diminished in HDCC to ensure good fiber dispersion and thus a satisfactory strain capacity with a strain-hardening phenomenon (Yu et al., 2017, 2018; Ding et al., 2018; Ji et al., 2019). The elimination of coarse aggregate and the reduction in the sand amount consequently raised the amount of cement in HDCC. However, it is noticed that higher cement dosage in HDCC, on one hand, increased the manufacturing cost; on the other hand, led to soaring heat of hydration and lager autogenous shrinkage of HDCC, which adversely reduced the compressive and tensile performance of HDCC.



Thus, the aim of this paper is to utilize the RP to partially replace the cement in HDCC, which could significantly reduce the amount of cement. The impact of RP particles on mechanical performance of HDCC, including the tensile and compressive performance was then discussed. The microstructures of RP-amended HDCC, including the hydration heat, thermal gravimetric analysis, and morphology of polyethylene fiber, were also investigated.

TEST PREPARATION

Raw Materials and Mix Proportions

The HDCC was consisted of three parts: cementitious binder, aggregate, and reinforced fiber. 42.5R type I Cement, ground granulated blast slag (GGBS), and RP were used as the binder constituents for HDCC. GGBS and SF particles were considered as the pozzolanic components, which would accelerate the development of hydrated calcium silicate (C-S-H). RP was utilized to partly substitute the cement owing to its potential pozzolanic function (Liu et al., 2017). The silica sand with the particle diameter of 70-120 meshes was used as the aggregate. The sand/binder ratio was controlled at a low value to ensure the fiber dispersion. The gradation curves of cementitious materials and silica sand are given in Figure 1. The diameter and length of PE fiber are 24 μ m and 18 mm, respectively. The tensile strength and Young's modulus of PE fiber are 2,400 MPa and 100 GPa and its elongation at breakage is 2-3%. Finally, a high-range superplasticizer was effectively employed to preserve the mixture flowability at a relatively small water/cementitious binder ratio.

Tiny particles of silica fume, RP and GGBS could densify of the matrix by filling the large pores, strengthen sand-paste interface and thus enhance the matrix strength. RP particles were gathered from a recycled factory, which crushed the construction wastes including both concrete and clay bricks. 85% RP particles had a



FIGURE 2 | Morphology of RP particles.

size lower than 45 μ m, within which 42% particles were lower than 10 μ m. The fine particles were beneficial to the strength development of matrix because of the apparent filling effect (Yu et al., 2020).

Figure 2 shows the morphology of RP particles, which has the similar irregular shapes to that of GGBS. The thermal gravimetric analysis of RP shows a trough around $680-720^{\circ}$ C, exhibiting a decarbonization of CaCO₃ at this temperature range with the escape of CO₂.

The four mixture proportions of HDCC are given in **Table 1**. Three replacement ratios of RP to cement dosage up to 60% by mass (10, 30, and 60%) were employed. The water/binder ratio of all the four mixtures was fixed at 0.36. The mixture was named as HDCC-RP-A, where A denotes as the mass ratio of RP. The fiber mass in all the mixtures was 19.4 kg/m³, representing a 2% volume.

Experimental Preparation

A TAM air calorimeter was utilized to measure the hydration heat of HDCC matrix following the instruction of ASTM C1702-09a ASTM International (2008). A fixed volume of each matrix (around 0.1 L) mixed at room temperature was rapidly put into the ampule and kept in the chamber for 72 h. TGA/DTG analysis was conducted by Mettlery Toled TGA/SDTA 852 balance. Each dry-slice sample around 12 mg was ground into fine powder and heated from 20 to 1,000°C at a fixed rate of 10°C/min.

The full-range tensile stress-strain relationship of HDCC under uniaxial tension was determined by the plain dumbbellshaped specimen (as shown in **Figure 3**) according to JSCE specification ^[16]. The dimensions of dumbbell-shaped specimen are shown in **Figure 4** with a cross-section of 30 mm \times 13 mm and the gauge length of 80 mm. For each case from HDCC-Reference to HDCC-RP-60%, four dumbbell specimens were cast and tested to obtain their tensile properties at 28 days. The dumbbell geometry could ensure the distribution of majority cracks in the gauge length to allow a more reliable measurement of tensile strain. Cube specimens with edge length of 50 mm were prepared for the compression test at 28 days. The detailed

TABLE 1 | Mix properties of HDCC.

Cement (kg/m ³)	RP (kg/m ³)	GGBS (kg/m³)	Silica sand (kg/m ³)	Water (kg/m ³)	PE fiber (kg/m ³)	HRWR (kg/m ³)	W/Binder
600	-	400	600	360	19.4	5	0.36
540	60	400	600	360	19.4	5	0.36
420	180	400	600	360	19.4	5	0.36
240	360	400	600	360	19.4	5	0.36
	Cement (kg/m ³) 600 540 420 240	Cement (kg/m³) RP (kg/m³) 600 - 540 60 420 180 240 360	Cement (kg/m³) RP (kg/m³) GGBS (kg/m³) 600 - 400 540 60 400 420 180 400 240 360 400	Cement (kg/m³)RP (kg/m³)GGBS (kg/m³)Silica sand (kg/m³)600-40060054060400600420180400600240360400600	Cement (kg/m³)RP (kg/m³)GGBS (kg/m³)Silica sand (kg/m³)Water (kg/m³)600-40060036054060400600360420180400600360240360400600360	Cement (kg/m³)RP (kg/m³)GGBS (kg/m³)Silica sand (kg/m³)Water (kg/m³)PE fiber (kg/m³)600-40060036019.45406040060036019.442018040060036019.424036040060036019.4	Cement (kg/m³)RP (kg/m³)GGBS (kg/m³)Silica sand (kg/m³)Water (kg/m³)PE fiber (kg/m³)HRWR





experimental program of compressive and tensile tests is listed in Table 2.

Experimental Setup and Procedure

Figure 4 shows the setup of uniaxial tensile test. The alignment of whole setup was specially adjusted before loading to avoid the eccentric loading. The tensile loading speed was kept at 0.5 mm/min according to Japan Society of Civil Engineers [JSCE] (2008) with a 50 N

TABLE 2 | Test program of HDCC with different replacement ratios of RP.

Mixture proportions	Compressive test	
	test	Tensile
HDCC-RP-Reference to HDCC-RP-60%	5	4
	5	4







preloading. The loading capacity of test machine is 50 kN. A light-weight frame holding two symmetrically installed transducers (LVDTs) was attached to the dumbbell-shaped

specimen to determine the displacement of specimen under tension. The tensile strain capacity of HDCC was calculated by the average reading of two LVDTs.



RESULTS AND DISCUSSION

Isothermal Calorimetry

The isothermal calorimetry was utilized to measure the hydration evolution of HDCC matrices under different RP contents, i.e., the replacement ratio from 10 to 60% of cement. **Figure 5** shows the normalized heat flux under unit weight of binder material. The addition of RP accelerates the hydration rate and it is demonstrated that the dormant period of HDCC-RP-60% matrix was shortest due to the existence of CaCO₃ as the main compound of RP, which is considered to act as the accelerating seeds to the formation of tricalcium silicate (C₃S) (Nehdi et al., 1996). The hydration activity of the four HDCC matrices mainly happened in the first 72 h, after which it decreased generally with curing age. The heat flux peak of HDCC matrices generally declined with the RP content (**Figure 5**), while the peaks of RP-10% and RP-30% were comparable to or even higher than that of the reference matrix.

Phase Development Analysis

The DTG results of HDCC matrices with different RP replacement ratios at 28d were shown in **Figure 6**. Three





significant peaks could be observed: the first peak is around 200°C, mainly related to the dehydration of ettringite; the second peak around 450°C is the dehydroxylation of $Ca(OH)_2$; and the third peak around 500–800°C is the decarbonization of $CaCO_3$ (Huang et al., 2017).

The peaks of $CaCO_3$ in different matrices increased significantly with the larger amount of RP, while the peak of $Ca(OH)_2$ of HDCC-RP-60% was lower than those of HDCC-Reference and HDCC-RP-30% at 28d due to the significant reduction in cement and the potential pozzolanic reaction of RP in HDCC-RP-60%.

Compressive Properties of HDCCs

Figure 7 displays the compression strength of HDCCs at 28d. The average strength value of reference specimen was 55.23 MPa and it raised to 66.0 MPa at the replacement ratio of 30%. The strength evolution of RP-amended HDCCs indicated that an appropriate amount of RP would promote the compression strength owing to its pozzolanic and filler effect. However, excessive RP, which meant an excessive replacement of cement up to 60% would damage the compressive strength to some extent and compressive strength of HDCC-RP-60% is 58.45 MPa.

Tensile Stress-Strain Curves and Tensile Properties

Figure 8 displays the tensile stress-strain relationships of HDCCs with different RP replacement ratios. The HDCC specimens in four cases show a robust strain-hardening performance. The tensile peak stress of HDCC-Reference was 7.00 MPa, which was close to the value of HDCC-RP-10% but lower than that of HDCC-RP-30% (i.e., 7.57 MPa). Moreover, the addition of RP did not sacrifice the tensile strain capacity of RP-HDCCs even up to the RP content of 60%.

Figure 9 shows the crack distribution of HDCC specimens at the unloading stage. The average crack numbers are 37, 42, 43, and 48 for HDCC-RP-Reference, HDCC-RP-10%, HDCC-RP-30%, and HDCC-RP-60%, respectively. The cracks evenly spread along the gauge length with an average crack spacing of 2.16 mm for HDCC-RP-Reference, 1.86 mm for HDCC-RP-30%



and 1.67 mm for HDCC-RP-60%, respectively. The mixing of RP boosted the crack development leading to a larger crack number and consequently a smaller the crack spacing, which may be due to the increase in interfacial stress between fiber and matrix.

The tensile parameters including the initial cracking stress σ_{tc} , peak stress σ_{tu} , strain capacity and strain energy g_{se} are shown in **Figure 10**. Detailed definitions of these parameters could be referred to Ding et al. (2018).

Figure 11 summarizes the tensile parameters of HDCCs under different replacement ratios. The peak stress σ_{tu} increased generally from 7.0 MPa of HDCC-RP-Reference to 7.09 MPa of HDCC-RP-10%, and then to 7.57 MPa of HDCC-RP-Reference-30%. It is noticed that the σ_{tu} of HDCC-RP-60% (6.66 MPa) was only 5.5% lower than that of the HDCC-RP-Reference. The high replacement ratio of cement by RP did not compromise the strength development of HDCC, but significantly increased its greenness and reduced its impact on the environment. The initial stress σ_{tc} shared a similar tendency with the σ_{tu} of HDCCs (see in **Figure 11A**).

The addition of RP had positive influence on the tensile strain capacity up to the replacement ratio of 30% due to the



enhancement of interfacial stress (see in **Figure 11B**). However, a further increase of the RP content up to 60% slightly weakened the strain capacity of HDCC-RP-60%, which could be attributed to the smaller crack width in HDCC-RP-60% (see in **Figure 11E**). The strain energy g_{se} , kept almost constant to HDCC-RP-30% at around 430 kJ/m³ and then decreased apparently to 335 kJ/m³ of HDCC-RP-60% due to the combination of lower σ_{tu} and ε_{tu} . The

 g_{se} of HDCC was significantly greater than those of conventional fiber-reinforced concrete and ultra-high performance concrete at the range of 50–100 kJ/m³ (Wille et al., 2011).

In addition, it is interpreted from **Figures 11D,E** that the reduction in ε_{tu} of HDCC-RP-60% was the result of finer crack width of RP-amended HDCCs. The RP particle size was smaller than that of cement particle, which would help to establish a



denser matrix, leading to tinier crack width. It is calculated that the average crack width of HDCC-RP-60% was around 120 μ m, which was close to that of PVA-ECC (Li, 2003).

Morphology of Fiber in HDCC

Figure 12 shows the morphology of fibers and fiber/matrix interface of HDCC-RP-Reference and HDCC-RP-30% by scanning electron microscope (SEM) observation. The SEM samples were taken from the ruptured surface of the dumbbell specimens. The majority of PE fibers were stretched out of the HDCC matrix with a blunt end (see in **Figure 12A**) or slightly damaged (see in **Figure 12C**). However, the fibers of HDCC-RP-30% were damaged more severely and led to a higher tensile strength (see in **Figure 12D**).

CONCLUSION

The impact of RP on hydration, phase development, compressive strength and tensile properties of HDCC were studied in the present research. Three replacement ratios of RP ranging from 10, 30 to 60% were employed. The detailed conclusions are listed as follows.

(1) The addition of RP accelerates the hydration rate and it is demonstrated that the dormant period of HDCC-RP-60% matrix was shortest due to the existence of $CaCO_3$ as the main compound of RP, which is considered to act as the accelerating seeds. The peaks of $CaCO_3$ in different matrices increased significantly with the larger amount of RP. However, the peaks

of Ca(OH)₂ decreased with higher RP contents due to the significant reduction in cement.

(2) All the HDCC mixtures demonstrated a robust strainhardening performance with evenly distributed cracks. The peak tensile stress and compressive strength of HDCCs increased with RP replacement ratio up to 30% due to a combination of pozzolanic and filler effect and these strengths decreased afterward. The crack number increased with RP contents, while the strain capacity of HDCC-RP-60% decreased slightly due to its the tinier crack width. The morphology of PE fiber of the tested specimen was investigated by the SEM analysis. The optimal replacement ratio of RP for future practical application is suggested for 30% provided with the similar particle size of current research, when the mechanical performance is considered.

DATA AVAILABILITY STATEMENT

The raw data supporting the conclusions of this article will be made available by the authors, without undue reservation.

AUTHOR CONTRIBUTIONS

JJ, HS, and LJ were responsible for designing the experiment scheme, test execution, data analysis, and writing the original manuscript. HR was responsible for guiding the experiment scheme and revision of the manuscript. YZ and YL participated in the experimental scheme design. All authors contributed to the article and approved the submitted version.

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Conflict of Interest: The authors declare that the research was conducted in the absence of any commercial or financial relationships that could be construed as a potential conflict of interest.

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