



Seagrass Meadows Reduce Wind-Wave Driven Sediment Resuspension in a Sheltered Environment

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Seagrass meadows are prominent in many coastal zones worldwide and significant contributors to global primary production. The large bottom roughness (or canopy) created by seagrass meadows substantially alters near-bed hydrodynamics and sediment transport. In this study, we investigate how a seagrass meadow in a lowenergy environment (forced by local winds) modifies near-bed mean and wave-driven flows and assess how this relates to suspended sediment concentration (SSC). A twoweek field study was conducted at Garden Island in southwestern Australia, a shallow and sheltered coastal region subjected to large diurnal sea-breeze cycles, typical of many low-energy environments where seagrasses are found. The mean and turbulent flow structure, along with optical estimates of SSC, were measured within both a seagrass canopy and over an adjacent bare bed. Near-bed mean current velocities within the seagrass canopy were on average 35% of the velocity above the canopy. Oscillatory wave velocities were less attenuated than mean current velocities, with nearbed values on average being 83% of those above the canopy. Mean and maximum shear velocities inferred from currents and waves above the canopy frequently exceeded the threshold for sediment resuspension, but no significant variation was observed in the SSC. However, a significant correlation was observed between SSC and bed shear stress estimated using near-bed velocities inside the canopy. When sediment was resuspended, there were substantial differences between the SSCs within and above the canopy layer, with higher levels confined within the canopy. This study demonstrates the importance of measuring near-bed hydrodynamic processes directly within seagrass canopies for predicting the role seagrass meadows play in regulating local rates of sediment resuspension.

Keywords: canopies, sediment transport, sea-breeze, seagrass, wind waves

INTRODUCTION

Seagrass meadows are a foundation ecosystem composed of marine angiosperms (flowering plants). They serve as a nursery habitat for fish and shellfish, act as coastal carbon sinks, contribute to the export of biomass in coastal zones, and enhance biodiversity (Duarte and Cebrián, 1996; Green and Short, 2003). Seagrasses are found in all continents except for Antarctica (Duarte, 2002) and

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1

are located in a range of hydrodynamic environments, from highly exposed to sheltered environments. Despite their global prevalence, few species possess strongly reinforced fibers that enable them to tolerate exposure to energetic swell conditions (Carruthers et al., 2007). Consequently, higher species richness and biomass is usually found in sheltered environments such as sounds, bays, peninsulas, and the leeward sides of reefs and islands where extreme wave conditions during storms are less frequent or absent (Lee Long et al., 1993; Copertino et al., 2016). While these regions tend to be sheltered from large swell waves, they can still be subjected to locally generated wind-waves and currents, which can drive daily variations in turbidity and ultimately light availability within the water column (Lawson et al., 2007).

Seagrasses require some of the highest light levels of any plant group worldwide (around 25% of incident radiation, Dennison et al., 1993) and light availability can often be a limiting factor for seagrass distribution and growth (Duarte, 1991). In coastal waters, light composition and intensity can be strongly influenced by turbidity caused by the resuspension of sediment from the seabed (de Boer, 2007). The above-ground structure formed by seagrass leaves and stems are known to substantially attenuate near-bed flows by exerting drag forces on the flow. This flow attenuation can reduce local rates of sediment resuspension and increase light availability, creating a feedback between seagrass growth and sediment/light in seagrass meadows (Adams et al., 2016); however, the detailed processes that govern these sediment transport processes have not been well quantified in field settings. Similar feedbacks have also been proposed for other benthic canopies such as coral reefs (Pomeroy et al., 2017); however, in contrast to those benthic environments, the flexibility of seagrass canopies results in a canopy whose geometry and flow resistance is dynamic.

The flexibility of seagrass affects how a meadow interacts with the overlying flow (Abdolahpour et al., 2018), as well as the near-bed flows in the meadow that may influence sediment resuspension. Seagrass flexibility and motion under wave-driven flows means that the canopy geometry and the drag forces exerted on the flow vary in time. When the seagrass blades are in their most upright position, drag imposed on the flow reaches its maximum value, as it is generated over the maximum canopy extent (Luhar and Nepf, 2011). However, when the seagrass bends, the drag on the flow reduces above as well as within the canopy (Abdolahpour et al., 2018). The reduction in drag as the blades bend, often during periods of strong currents, has important ecological effects such as the prevention of uprooting that would otherwise result in seagrass loss (Vogel, 1984).

A number of field studies have investigated the impact of seagrass on hydrodynamic processes over a range of conditions, water depths and coastal environments, such as open coastlines (Gacia and Duarte, 2001; Infantes et al., 2012), sheltered macrotidal environments (Koch and Gust, 1999; Hasegawa et al., 2008; Gruber and Kemp, 2010) and shallow bays (van Keulen and Borowitzka, 2002; Lawson et al., 2007; Gruber and Kemp, 2010; Hansen and Reidenbach, 2012; Reidenbach and Thomas, 2018). While these studies have helped explain how a seagrass meadow attenuates near-bed flows, and by inference turbidity and sediment resuspension, the direct connections between the hydrodynamics within a canopy and how these processes ultimately determine suspended sediment concentrations (SSCs) remain poorly quantified.

In this study, we use *in situ* measurements to investigate how the hydrodynamics are modified within a seagrass canopy in a sheltered, wind-wave driven environment and how they influence sediment resuspension. The objectives of this study are to (1) quantify how the steady and oscillatory (wave) components of the flow in low energy hydrodynamic environments are affected within seagrass canopies; and (2) evaluate how the modification of the hydrodynamics within a seagrass canopy influences sediment resuspension.

BACKGROUND: FLOW-SEAGRASS INTERACTIONS

Wind blowing over a water surface transfers momentum into the water column leading to the generation of mean currents. If the seabed has small bottom roughness (e.g., a bare sandy bed), these currents form a logarithmic velocity profile (Raupach et al., 1991) in a bottom boundary layer adjacent to the seabed where turbulent shear stresses are approximately constant (**Figure 1A**). Under these conditions, turbulent shear stresses within the bottom boundary layer are transmitted to the seabed where they exert forces that mobilize and transport sediment.

Many coastal environments are characterized by having much larger bottom roughness than a bare sandy bed. This roughness can range from boulders and rocky substrata to canopies formed by seagrass meadows, coral reefs, mangroves and kelp forests. These canopies modify the vertical flow structure near the bed, attenuating the mean currents and wave velocities, which alters the distribution of turbulent shear stresses within the water column (Ghisalberti and Nepf, 2002; Nepf, 2012). When mean currents interact with seagrass meadows (and canopies in general), the largest shear stresses $\tau_{c,rough}$ occur at the top of the canopy due to the vertical gradient in velocity created by the drag discontinuity at that height (Luhar and Nepf, 2011; Abdolahpour et al., 2018; Figure 1B). Note that a summary of all notations that will be used in this paper is provided in Table 1. The canopy drag forces then attenuate the in-canopy flow, which reduces the shear stresses exerted on the bed $(\tau_{c,bed})$. Dimensional reasoning suggests that when spatially averaged, this bed shear stress is related to the (i) canopy density, (ii) canopy/submergence depth and (iii) the contribution of wake turbulence to the near-bed flow (Nepf, 2012).

If the wind is consistent and the fetch sufficiently large, locally generated wind-waves can drive oscillatory flows. These flows also interact with the bed to form a thin wave boundary layer (WBL) (**Figure 1C**). Due to the oscillatory nature of the flow, this WBL has variable thickness, which is a function of the maximum shear velocity within a wave cycle $(u_{*,max})$ and the wave angular frequency (ω) (Grant and Madsen, 1979). We note that the shear velocity (u_*) and shear stress (τ) are used interchangeably in this paper as they are related based on seawater density ρ_w through $u_* = \sqrt{\tau/\rho_w}$. In addition to the bottom WBL,



growth of this WBL is limited when a seagrass canopy of sufficient density is present (D). Additionally, a wave-driven shear stress is created at the top of the canopy ($\tau_{w,rough}$) due to its interaction with the waves.

wave-generated flows that interact with a canopy create another region of shear at the top of the canopy (**Figure 1D**), due to drag imposed by the canopy. As a consequence of these mean shear stresses, wave-driven mean currents can be generated within the meadow (Luhar et al., 2010; van Rooijen et al., 2020). Due to how wave-driven pressure gradients interact with canopy drag and inertial forces, the oscillatory flows generated by waves are less attenuated within a seagrass canopy when compared to an unidirectional flow, with high-frequency waves also less attenuated than longer period swell waves (Lowe et al., 2005; Nepf, 2012). As sheltered, sea-breeze-dominated environments are more likely to be dominated by locally generated waves, the high-frequency waves portion of the spectrum tends to be more important for sediment resuspension (Lawson et al., 2007).

The nonlinear interactions between waves and currents largely present in field measurements influences the shear stresses exerted on a bed, which consequently affects how sediment is resuspended. Wave-current interactions result in bed shear stresses that are larger than a simple vector summation of the contributions from the current ($\tau_{c,bed}$) and wave motions ($\tau_{w,bed}$) (Grant and Madsen, 1979; Shaw and Trowbridge, 2001; Soulsby and Clarke, 2005). Thus, it is common to represent bed stresses by their mean (τ_m) and maximum (τ_{max}) values. τ_m represents the average stress over a wave cycle due to waves and currents, and has been suggested to be important for the diffusion of sediment into the flow outside the WBL, thus controlling the shape of the SSC profile (Soulsby and Clarke, 2005). τ_{max} , representing the maximum stress over a wave cycle due to waves and currents, often controls the grain sizes capable of being suspended from the bed (Grant and Madsen, 1979; Soulsby and Clarke, 2005; Malarkey and Davies, 2012).

Understanding the interactions of waves and currents with seagrass canopies is critical to predict sediment resuspension in environments where canopies are present. Due to the challenges

TABLE 1	Summary	of the	notation	used	within	this	manus	script
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Variable	Definition				
α	Flow attenuation within the seagrass meadow				
A	Wave orbital excursion				
δ	Wave boundary layer thickness				
d	Distance from bottom				
Dx	Grain diameter for which $x\%$ of the sediment is smaller than the total sample				
γ	Attenuation correction factor				
E	Spectral density				
f	Wave frequency				
f _{wr}	Wave friction factor				
k	Wave number				
ρs	Sediment density				
ρ_W	Water density				
ϕ	Angle between waves and currents				
τ	Horizontal shear stress				
$U_* = \sqrt{\tau/\rho_W}$	Shear velocity				
ū	Time-averaged velocity				
U _{br}	Representative bottom-orbital velocity derived from the velocity spectrum				
Urms	Root-mean-squared oscillatory (wave) velocity				
ω	Wave angular frequency				
Z ₀	Hydraulic roughness				
Subscripts					
bed	At the bed				
С	Current alone				
can	In-canopy				
cr	Critical				
i	Wave frequency component				
m	Mean (for shear stress and shear velocity)				
max	Maximum (for shear stress and shear velocity)				
rms	Root mean square				
rough	At the top of canopy elements				
W	Wave alone				
∞	Free stream velocity				

with measuring these processes in situ in the field, studies have tended to focus on those (1) derived under controlled flume experiments, using physical models that aim to mimic the canopy with idealized forcing conditions (Chen et al., 2007; Yang and Nepf, 2019; Marin-Diaz et al., 2020); or (2) using field measurements taken from above the meadow and predicting the hydrodynamic-sediment transport processes in the canopy (e.g., Hasegawa et al., 2008; Bradley and Houser, 2009; Infantes et al., 2012). However, as the seagrass modifies the vertical structure of the flow, for many canopy systems the bed shear stresses are often much smaller than the shear stresses at the top of the canopy $(\tau_{\textit{bed}} \ll \tau_{\textit{rough}})$ (Le Bouteiller and Venditti, 2015). Therefore, inferring sediment resuspension in the presence of canopies from depth-averaged velocities measured above the roughness layer and applying a friction coefficient tends to overestimate sediment transport. This has been supported by flume experiments (Bouma et al., 2007; Le Bouteiller and Venditti, 2015) and in the field (Bouma et al., 2007; Pomeroy et al., 2017), for studies of both aquatic vegetation and over coral reef canopies.

There is a need for field observations of sediment transport processes in seagrass meadow that include in-canopy measurements to fully assess how flow modifications by the canopies locally affect sediment resuspension in these coastal environments. Near-bed measurements within the seagrass meadow calculating the wave and current contribution to bed shear stress are not common, with some exceptions (see Reidenbach and Thomas, 2018). Following the methodological approach by Pomeroy et al. (2017), who studied suspended sediment transport over a coral reef canopy, this paper investigates how seagrass meadows modify hydrodynamic and sediment transport processes using direct in-canopy near-bed measurements.

MATERIALS AND METHODS

Study Site

To quantify flow attenuation and sediment dynamics within a sheltered seagrass meadow, we identified a site where: (1) both a bare bed and a seagrass canopy were in close proximity, (2) there was minimal incident swell with a small tidal range (i.e., a low energy environment) and (3) the forcing (in this case due to the sea-breeze) was highly variable over 2 weeks, such that different conditions could be evaluated. The selected study site was located on the eastern side of Garden Island in Cockburn Sound, approximately 30 km south of Perth, Western Australia (Figure 2). The shallow and sheltered environment found in Garden Island is representative of many other seagrass sites around the world, where there is often greater diversity of seagrass species when compared to sites exposed to energetic swell (Lee Long et al., 1993; Copertino et al., 2016). The study site provides suitable habitat for the abundant and dense seagrass patch found there, with 10 different species of seagrasses being found within the Sound, mostly dominated by the Posidonia genus (Silberstein et al., 1986).

Garden Island, along with offshore reef systems, mostly shelters the site from offshore swell waves. The study site experiences a micro-tidal regime with a mean range of only 0.4 m (maximum spring tide range 0.6 m), which results in low tidal currents within Cockburn Sound of between 0.05 and 0.1 m s⁻¹ (Ruiz-Montoya and Lowe, 2014). Along this section of the Western Australian coast, the presence and location of atmospheric pressure troughs and their interaction with geostrophic offshore winds drive one of the strongest and most persistent sea-breeze systems in the world. While the mean sea-breeze speed is around 8 m s⁻¹, it frequently exceeds 15 m s⁻¹ and can be as high as 20 m s⁻¹, particularly during the summer months (Masselink and Pattiaratchi, 2001). This sea-breeze cycle, combined with the low tidal range and the attenuation of swell waves, often results in strong daily cycles of the hydrodynamic (Gallop et al., 2012) and sediment transport processes (Masselink and Pattiaratchi, 2001).

Measurements

A 2-week field experiment (16–31 October 2015) was conducted using an instrument frame (**Figure 3**) deployed sequentially over



approximate region shown in (B).



a dense seagrass meadow and then a sandy bed. The instrument frame was first deployed over a dense *Posidonia* seagrass meadow (the "canopy" site) for 7 days. The seagrass canopy at the site had a shoot density of 400 ± 88 shoots m⁻² and average height of 35 ± 3 cm when fully upright. This study was part of a larger experiment, with detailed properties of the seagrass meadow reported in Olsen et al. (2018). After the first 7 days, the instrument frame was then moved approximately 40 m to an area with no seagrass (the "bare bed" site). The water depth at both sites was ~1.5 m. The proximity of these two sites enabled us to isolate the impact of the seagrass canopy on the hydrodynamics and sediment transport processes.

A Sea-Bird Eco FLNTU Optical Backscatter Sensor (OBS) and a Nortek Vector Acoustic Doppler Velocimeter (ADV) were positioned \sim 10 cm above the bed (cmab), which sampled within the seagrass canopy and will be hereafter referred to as the "bottom" ADV and OBS (**Figure 3**). A second pair of ADV and OBS was placed \sim 35 cmab (hereafter referred to as "top"), which was the height of the seagrass canopy when fully extended. Each ADV measured horizontal and vertical velocities at 8 Hz for 2048 s (\sim 34 min) while the OBS sampled at 1 Hz for 20 s every 5 min. At both deployment locations, current velocity profiles were measured with an upward facing Nortek Aquadopp Profiler with High Resolution mode enabled

(ADP) that was positioned on the bed alongside the other instrumentation and sampled continuously at 1 Hz with a cell size of 4 cm. We carefully trimmed a small patch of \sim 15 cm diameter (roughly the ADV probe size) of seagrass blades around the ADV, ADP and OBSs. This procedure is widely used (e.g., Hasegawa et al., 2008; Abdolahpour et al., 2017; Reidenbach and Thomas, 2018) to minimize potential interference with the instruments. The interference is expected to be small, as the study site is situated well within a dense meadow, so that the flow would still be affected by the surrounding seagrass. High frequency wind waves were expected to dominate at this site but also more attenuated with depth. To ensure we captured the high frequency wind waves, we located the ADV canisters with pressure sensors at the top of the frame (~ 120 cmab). Wind data used in this study were extracted from the Garden Island Weather Station (Figure 2), located less than 7 km from the deployment site and managed by the Australian Bureau of Meteorology.

Hydrodynamic Data Analysis

Pressure time series data recorded by the ADVs were used to quantify wave conditions. Each burst of data was split into two ensembles of 8,192 samples (~17 min). Pressure spectra were then determined by applying a Fast Fourier Transform with a 512-point Hamming window to the timeseries, which were then converted to wave (surface elevation) spectra using linear wave theory. From the wave spectra, the significant waveheight $(H_{m0} \approx 4\sqrt{m_0})$ and the mean wave period $(T_{m02} = \sqrt{m_0/m_2})$ for the wind-sea (0.125–1 Hz) and swell waves (0.05–0.125 Hz) were calculated separately from the *n*th-order spectral moment m_n through integration of the energy within these respective bands.

The raw ADV and ADP velocity measurements were initially filtered based on low signal correlation with the threshold (60%) recommended by McLelland and Nicholas (2000). Velocity spikes in the remaining data caused by debris and seagrass blades that entered the sample volume were removed with a kernelbased algorithm (Goring and Nikora, 2002). To calculate the attenuation of the flow inside the canopy, as well as an estimate of the bed shear stress exerted on the sediment, velocity data was decomposed into a fluctuating (wave and turbulence) velocity component and a mean current component. The wave velocity direction was calculated from the covariance matrix applied to the fluctuating velocity component (Emery and Thompson, 2001), $C = cov (x - \overline{x}, y - \overline{y})$, where x and y are the velocities measured by the instruments in relation to its own compass and the overbar represents the time-averaged velocity. Within each burst of data, the wave angle relative to the instrument is defined as:

$$\phi_{w} = \frac{1}{2} \arctan\left(\frac{2[\cos\left(x - \overline{x}, y - \overline{y}\right)]}{\cos\left(x - \overline{x}, x - \overline{x}\right) - \cos\left(y - \overline{y}, y - \overline{y}\right)}\right)$$
(1)

and the current angle is defined as:

$$\phi_c = \arctan\left(\frac{\overline{y}}{\overline{x}}\right) \tag{2}$$

For each burst, the mean velocity component was removed and the remaining oscillatory (wave) component was rotated into a coordinate system using ϕ_w so that the maximum velocity variance within the burst was along the principal (major) axis. The root-mean-squared wave velocity (u_{rms}) in every measurement cell over the water column was then estimated from the velocity spectra derived from the rotated data:

$$u_{rms} = \sqrt{2\sum_{i} (S_{xx,i} + S_{yy,i}) \Delta f_i}$$
(3)

where S_{xx} and S_{yy} represent the spectral densities of the horizontal velocities after removing the mean velocity for each burst and f_i represents the *i*th frequency component (from 0.05 to 4 Hz for the ADVs and from 0.05 to 0.5 Hz for the ADP).

To quantify the attenuation of the flow inside the canopy, we calculated the attenuation parameter α (Lowe et al., 2005), which represents the ratio between the flow velocities within the canopy region and the flow velocities above the canopy. The time-averaged value for the mean velocity component case, α_c is the simple ratio using the 15 min-averaged velocities. To evaluate the wave attenuation due to the canopy interaction for each frequency, we first determined a correction factor that accounted for the natural decay of wave orbital velocities below the free surface (i.e., independent of canopy drag), as predicted from linear wave theory:

$$\gamma_i = \frac{\cosh (k_i d_\infty)}{\cosh (k_i d_{bed})} \tag{4}$$

where k_i is the wave number of the *i*th component of the wave spectrum; *d* is the distance from bed; subscripts *bed* and refers to the measurements close to bed and at a measurement location above the canopy, respectively. Thus, for the wave attenuation, the α_w value was determined as the ratio between $u_{rms,bed}$ (correcting the attenuation accordingly for each frequency) and the free stream (u_{rms} .):

$$\alpha_{w} = \sqrt{\frac{\gamma_{i}^{2} * 2\sum_{i} (S_{xx,i,bed} + S_{yy,i,bed}) \Delta f_{i}}{2\sum_{i} (S_{xx,i,\infty} + S_{yy,i,\infty}) \Delta f_{i}}}$$
(5)

To quantify the mean and maximum stresses at the bed $(\tau_{m,bed} \text{ and } \tau_{max,bed})$ as well as in the roughness layer $(\tau_{m,rough})$ and $\tau_{max,rough}$ both with and without the canopy, we used the approach proposed by Soulsby and Clarke (2005) as adapted by Malarkey and Davies (2012). The differences between Soulsby and Clarke (2005) and Malarkey and Davies (2012) and other models such as Grant and Madsen (1979) and Madsen (1994) are: (1) the current velocity at the interface of the boundary layer and the overlying fluid is assumed to be steady; (2) iteration is not required to find a solution as the effective (total) friction velocity and WBL thickness are not calculated from the combined wave and current quantities, but rather is inferred from a wave-only friction velocity; and (3) the oscillatory component of the stress is not enhanced by the current; thus, τ_m and τ_{max} can be calculated independently. In the Soulsby and Clarke (2005) method, the

shear stresses τ_m and τ_{max} are calculated by superposition of the waves and currents at an angle ϕ , using: the water depth (*h*), the seabed roughness, the near-bed wave velocity amplitude (representative bottom orbital velocity $u_{br} = u_{rms}$), wave period (T) and the depth-averaged current (\overline{u}). The key adaptation made to the Soulsby and Clarke (2005) method by Malarkey and Davies (2012) was that instead of \overline{u} , the calculation considers u(d). This enables point measurements at height d to be used in the analysis. While τ_m determines the friction imposed on currents and contributes to the diffusion of sediment into the upper flow, τ_{max} is responsible for the initiation of the sediment motion as well as near-bed diffusion. τ_m is expected to be greater than τ_c , and τ_{max} to be greater than the vector sum of τ_c and τ_w (Soulsby and Clarke, 2005). These methods have been successfully applied to estimate mean and maximum bed shear stresses accounting for the attenuation of the near-bed flows by the canopy, including coral reefs (Pomeroy et al., 2017, 2021), saltmarsh (Chen et al., 2020) and benthic feeding tubes (Egan et al., 2020, 2021).

To calculate a hydraulic roughness (z_0) for the canopy site, we followed the methodology described by Pomeroy et al. (2017), who considered $z_0 = z_{0a}$, the hydraulic apparent bed roughness due to wave-current interaction. For every 15 min of data, we evaluated the logarithmic fit with the mean velocity data through the water column (starting at the top of the profile and sequentially adding the bottom cells until the fit deviated substantially, as defined by linear regression). We observed that the best fit was consistently found for the region that coincided with the top of the canopy (~0.35 m) up to ~5th highest cell (d = 0.92 m). Applying the "law of the wall" (Raupach et al., 1991) to that region, we then calculated $u_{*c,rough}$ and z_0 . Conversely, to estimate the bed shear stress at the bare site where the bed roughness was governed by sandy bed sediment, we defined $z_0 = 2.5D_{50}/30$.

We next used the Malarkey and Davies (2012) set of equations to calculate the mean stress τ_m as well as the maximum stress τ_{max} :

$$\tau_{max} = \sqrt{\left[(\tau_m + \tau_w |\cos \phi|)^2 + (\tau_w |\sin \phi|)^2 \right]}$$
(6)

We calculated the angle ϕ as the absolute angle difference between ϕ_w and ϕ_c . To calculate τ_w , a wave friction factor f_{wr} needs to be specified. We used the formulation proposed by Lawson et al. (2007), which accounts for different wave frequencies:

$$f_{wr} = \begin{cases} 0.04 \left(\frac{A}{k_s}\right)^{-\frac{1}{4}} \text{ for } \frac{A}{k_s} \ge 100\\ 0.4 \left(\frac{A}{k_s}\right)^{-3.4} \text{ for } 10 \le \frac{A}{k_s} < 100 \\ 0.071 \left((0.4) \ 10^{-\frac{3}{4}}\right) \text{ for } \frac{A}{k_s} < 100 \end{cases}$$
(7)

where $A = u_{br}T_{m02}/2\pi$ is a representative wave orbital excursion, $k_s = 3D_{84}$ and D_{84} is the grain diameter for which 84% of the sediment is smaller. Finally, in our application of the Malarkey and Davies (2012) formulation, we defined the WBL thickness (δ) in terms of wave-alone friction velocity u_{*w} and a time-invariant eddy

viscosity. The lower limit of the WBL thickness was set to be $\delta > D_{50}$, as the WBL cannot be thinner than the diameter of the bed sediment. Inside the WBL the eddy viscosity was a function of the effective shear stress, while outside the WBL the eddy viscosity was calculated following Grant and Madsen (1979).

We note that there were periods where the bottom ADV data included spikes, due to occasional interference with moving seagrass blades, that resulted in a time series with several gaps. As a consequence, data from the bottom ADV was only used to quantify the mean currents within the canopy. To estimate $u_{*,bed}$ we instead used data from the first cell of the ADP, which was located ~21 cmab but still within the seagrass canopy.

Sediment Characterization

We collected and analyzed three \sim 500 g superficial (between 5 and 7 cm) bed sediment samples from each site. To obtain a grain size distribution, the bulk bed samples were first split using a riffle splitter and then subsampled into individual replicates with a micro riffle splitter. Particles greater than 2 mm were removed with a sieve. The samples were then immersed in a solution of 10 mL of alkaline sodium hexametaphosphate and 800 mL of deionised water, and ultrasonically stirred for 60 s before being subjected to laser diffraction analysis (Cheetham et al., 2008; Di Stefano et al., 2010) using a Malvern Mastersizer 2000 (Malvern Instruments Ltd., Malvern, United Kingdom). Laser diffraction analysis converts optical scatter from a sediment sample into a grain size distribution using a specified optical model. For sand-sized particles, the use of different optical models has been shown to have a negligible impact on the results (Blott and Pye, 2001). In this study we used Mie theory, which describes the propagation of the electromagnetic wave of light in space taking into account the light transmission (Di Stefano et al., 2010). In our study, we assumed a refractive index of 1.650.

Suspended Sediment Concentration

To understand the impact of hydrodynamics in sediment resuspension, a time-series of SSC was estimated from the OBS turbidity measurements. The OBS data was calibrated with 2 L water samples that were obtained at hourly intervals (over a number of days) during the experiment. The samples were collected using peristaltic pumps via intake ports collocated with the OBS instruments and oriented perpendicular to the dominant flow. In this low energy environment, inefficiencies or bias in particle capture are expected to be minimal (Bosman et al., 1987). The *in situ* samples were pumped through a LISST 100X particle size analyzer to obtain a suspended sediment grain size distribution, and then vacuum filtered onto pre-weighed membrane filters (Whatman ME27, 0.8 mm), dried (75°C for 24 h) and weighed to calculate the SSCs.

To relate the suspended sediment grain size distribution to the shear stresses above and within the canopy, we determined the equivalent grain size that could be suspended by a given shear velocity based on the downward particle fall velocity (w_s). We adopted the formulations by Soulsby (1997):

$$w_s = \frac{\nu}{D} (\sqrt{10.36^2 + 1.049 D_*^3} - 10.36)$$
(8)

with dimensionless grain size D_*

$$D_* = \sqrt{\frac{g(s-1)}{\nu^2}} D$$
 (9)

where *D* is the sediment grain size in suspension, v is the water kinematic viscosity at 20° (10⁻⁶ m²s⁻¹), *g* is the gravitational acceleration (9.8 m s⁻¹), *s* is the ratio of carbonate sediment grain density (assumed to be $\rho_s = 2$, 600 kg m⁻³) at the site to water density ($\rho_w = 1$, 026 kg m⁻³). As the state of suspension is not within the scope of this work, we assumed fully developed suspension, $w_s = u_*$, which in our analysis was then used to defined a critical velocity $u_{*,cr}$ to suspend the sediment from the bed (**Table 2**). If the inferred shear velocity calculated from Eqs 8 and 9, we expected to see a signal registered in the time series measured by the OBSs.

RESULTS

Forcing Conditions

The diurnal sea-breeze cycle, which varied in direction and magnitude throughout the day as well as over a number of days, was the dominant forcing throughout the field experiment. During a typical day, the sea-breeze cycle followed a similar pattern. In the morning, the wind direction ranged from 70 to 120° (easterly wind) and was low in magnitude (1–5 m/s). In the afternoon, the wind direction rotated anti-clockwise to a southerly shore-parallel wind (180–230°) and increased in magnitude (5–10 m/s). The peak wind speed typically occurred between 16:30 and 19:00 (**Figure 4A**).

The observed hydrodynamics varied in response to the seabreeze cycle (**Figures 4B–D**). On a weak sea-breeze day (e.g., 22nd and 28th October), the increase in wind speed between the morning and afternoon was generally less than 4 m/s (peak less than 7 m/s). On these days, there was minimal increase in current speed or wave height after the onset of the sea-breeze. In contrast, during a strong sea-breeze day (e.g., 20th October) when the wind speed exceeded 10 m/s, strong mean currents were generated that lagged the wind by 1–4 h. Daily variations in the wave spectra in response to the wind are also evident (**Figure 4D**). With the onset of the sea-breeze late in the morning and early in the afternoon, the peak wave energy on the day occurred within

TABLE 2 | Mean and maximum sediment size (D_{50}) for samples taken *in situ* in both bare bed and canopy sites.

	$\pmb{D_{50}}~(\mu \textit{m})$	$u_{*,cr}$ average(cm/s)	u _* , _{cr} range(cm/s)
Seagrass bed	160 ± 29	1.77	1.24-2.32
Bare bed	150 ± 20	1.58	1.24-1.95

higher frequencies (0.33–0.5 Hz; 2–3 s, gray line in **Figure 4E**), characteristic of local sea-breeze generated waves.

A persistent but weak swell wave (0.125 to 0.05 Hz) contribution was also observed throughout the experiment. Late at night or early in the morning when the sea-breeze diminished, a greater proportion of the wave energy was within this lower swell frequency range of the spectrum (around 0.05-0.07 Hz, or 14-20 s). A notable exception was the relatively large swell waves measured on the 19th October. During this event, the swell wave height (\sim 13 cm) was much larger than at other times during the experiment. A comparison between locally measured wave heights with offshore wave buoy data obtained from the Western Australia Department of Transport (not shown) indicate that these larger waves were related to larger offshore swell conditions ($H_{m0} = 2 \text{ m}$, $T_{m02} = 12 \text{ s}$) propagating to the study site within Cockburn Sound. In contrast, the three highest wave heights registered by the instruments (from the 25th to the 27th) were generated locally by strong winds, with short period waves $(T_{m02} = 2 \text{ s})$ reaching heights over 25 cm on the 25th and 27th after midnight in both cases. On these days, the wind did not follow the typical sea-breeze cycle due to other atmospheric factors or the sea-breeze directional pattern was unclear. Despite the dominant direction of waves from the south for most of the experiment, the bigger waves on those days originated from the east (Figure 4F). When winds are blowing from this direction, they have enough fetch (roughly 10 km) to develop and generate waves from 5 to 27 cm and periods that varied from 2.5 to 3 s (Figure 4D), which peaked in the early hours of the following day, e.g., 2 a.m. on the 25th October (25 cm) and 4 a.m. (27 cm) on the 27th October.

Influence of the Seagrass Canopy on the Hydrodynamics

The flow attenuation within the seagrass canopy varied over the experiment (Figures 5A,B). Although the free-stream values of the mean current and wave orbital velocities were often similar in magnitude during the experiment (up to ~ 10 cm/s), the mean current velocities within the canopy were much more attenuated than the wave velocities (Figure 5C). On average, mean current velocities within the canopy were much smaller than above the canopy (averaging $\alpha_c = 0.35$, Figure 5D), a value much lower than calculated based on the root-mean-squared of wave velocities (averaging $\alpha_w = 0.83$, Figure 5E). When comparing flow attenuation for different portions of the wave spectrum (not shown), we found that values were comparable to the bulk (rootmean-squared) values ($\alpha_w \sim 0.9$) for wave frequencies between 0.06 and 0.2 Hz, but slightly lower for very low frequencies (< 0.01 Hz) and very high frequencies (> 0.35 Hz, $\alpha_w \sim 0.7$). As natural wave attenuation over depth according to linear wave theory (i.e., for inviscid, irrotational waves) is accounted for in the attenuation parameter calculation (Eqs 4 and 5), the observed attenuation is thus due to the influence of canopy forces alone.

Sediment Analysis

The bed sediment within the seagrass canopy was comparable to, but slightly finer than, the sediment at the bare bed



site (e.g., canopy: $D_{50} = 440 \,\mu\text{m}$; bare bed: $D_{50} = 520 \,\mu\text{m}$; **Figures 6A,B**). At both sites, the grain size was approximately normally distributed. The grain sizes of the bed samples were considerably coarser than the suspended sediment grain sizes (Figures 6C,D). The grain sizes of the suspended sediment were comparable across both sites, although slightly coarser at the



attenuation parameter, α_w within the seagrass meadow. In (**D**,**E**), shades of green represent the frequency of data in each bin and white crosses represent average attenuation parameter values.

canopy site (e.g., canopy: $D_{50} = 160 \ \mu\text{m}$; bare bed: $D_{50} = 150 \ \mu\text{m}$; **Figures 6C,D**). There was little difference in the suspended sediment grainsize distribution for samples collected at different elevations at each site (not shown).

Influence of Hydrodynamics on Suspended Sediment Concentration Variability

Here we investigate how the presence of the seagrass canopy influences local SSCs relative to the bare bed. Mean bed shear velocities ($u_{*m,bed}$, light green time series in **Figure 7A**) were on average 40% of maximum bed shear velocities ($u_{*max,bed}$, red time series in **Figure 7B**). $u_{*m,bed}$ was always lower than the critical shear velocity required to suspend the sediment at both the seagrass and bare bed locations (**Figure 7A**), averaging 29% of the critical shear velocity. At the top of seagrass canopy, mean shear velocities ($u_{*m,rough}$, dark green time series in

Figure 7A) were also substantially lower than the maximum shear velocities ($u_{*max,rough}$, blue time series in **Figure 7B**) during the vast majority of the experiment, with $u_{*m,rough}$ on average 52% of $u_{*max,rough}$. At both locations (seagrass and bare bed), the shear velocities varied mainly in response to the wave changes. The peaks in SSC in the early hours of the 25th and 27th (**Figure 7C**) are preceded by peaks in the wave height governed by (local) high frequency waves seen in **Figure 4D**).

Mean and maximum bed shear velocities $u_{*m,bed}$ and $u_{*max,bed}$ within the seagrass canopy were always lower than the respective shear velocities, $u_{*m,rough}$ and $u_{*max,rough}$, measured at the top of the canopy. While $u_{*m,bed}$ represented an average of 60% of $u_{*m,rough}$, $u_{*max,bed}$ was on average 82% of $u_{*max,rough}$. $u_{*m,rough}$ and especially $u_{*max,rough}$ frequently overcame the critical shear velocity threshold (dashed line in **Figure 7**). If shear stresses measured at the top of the canopy exceeding threshold values were a local driver of sediment resuspension, we would



expect a strong correlation between SSC and either $u_{*m,rough}$ or $u_{*max,rough}$. However, we observed only minimal increases in SSC near the bed within the seagrass canopy when these shear stresses exceeded critical values (**Figure 7C**).

The shear velocity at the top of the canopy is based on the elevated shear stress the overlying water column would experience due to the canopy drag. The bed shear velocity, however, is based on the stress that would be exerted by the attenuated canopy flow on the sediment at the base of the seagrass canopy. For the bare bed experiment, $u_{*max,bed}$ well-exceeded the critical shear velocity on two occasions, with a direct effect being observed in the sediment concentration, which increased SSC by nearly an order of magnitude from baseline typical values (**Figure 7C**). During the seagrass canopy experiment, $u_{*max,bed}$ remained below the critical threshold, and consequently, SSC was always less than 10 mg/L, increasing by a maximum factor of 2 (from ~4 to 8 mg/L). However, during the bare bed experiment, SSC values peaked nearly 50 mg/L on the early morning of the 25th and 27th, when $u_{*max,bed}$ was appreciably higher than the critical shear velocity, increasing by a factor of 12 times (from \sim 4 up to 48 mg/L).

To explore the relationships between these different shear velocity definitions and SSC responses, we grouped the time series data into bins with common shear velocities (using a bin interval of 0.06 cm/s) and averaged the SSC values that occurred during these collections of times. This was done to average out the effects of any noise or influence of anomalous (isolated) events and focus on the dominant trends observed during the study. There is no relationship between shear velocities measured at the top of the canopy ($u_{*max,rough}$, i.e., that represents stresses exerted on the overlying water column) and the SSC, even when $u_{*max,rough}$ exceeded the estimated critical threshold ($u_{*,cr}$) for sediment transport ($R^2 = 0$ in **Figure 8**, purple dots). Bed shear velocities ($u_{*,max,bed}$) during the bare bed experiment (red dots



in **Figure 8**), also show no correlation with SSC under the critical shear velocity. These relatively low background values in SSC not driven by local sediment resuspension may be due to other particles in suspension or sediment advection that are not necessarily related to the local critical shear velocity. However, above the critical shear velocity for the bare bed, the maximum bed shear velocity and SSC are strongly correlated for the bare bed case ($R^2 = 0.99$). This strong correlation for the bare bed sediment resuspension is thus consistent with the critical threshold being exceeded, as calculated using Eqs 8 and 9.

As the seagrass canopy and bare bed experiments were conducted consecutively, the natural forcing conditions were not identical, and the number of times the estimated shear velocity overcame the critical shear velocity was lower when measuring at the seagrass canopy site. However, there were still enough events to verify that the canopy had an appreciable effect on reducing the SSC. On the early morning of the 17th and 20th, and late night of the 20th, when the seagrass canopy experiment was being conducted, the waves reached comparable heights to the 24th, 28th, and 29th (during the bare bed experiment). As a consequence, the sediment experienced similar values of $u_{*,max,bed}$ for the two experiments, however, SSC was lower in the canopy case than in the bare bed. For the same velocities above

 $u_{*,cr}$, the canopy SSC values were on average 82% of the bare bed values. While the $u_{*,max,bed}$ values did not substantially exceed the critical threshold $u_{*,cr}$ during within the seagrass canopy (green dots in **Figure 8**), even below this threshold the low SSCs were correlated with $u_{*,max,bed}$. This response is different from the bare bed case, and while the cause is unknown, may indicate that even below the critical threshold there is still a weak dependency of the SSC on the canopy flow; for example, due to weak resuspension by wake-generated turbulence.

At the canopy site, we also observed evidence of some attenuation of SSC with height above the bed (**Figure 9**). At this site, SSC at the top of the canopy (using a 2-h window moving average) was on average \sim 80% (± 11%) of that within the canopy. Over the bare bed, the equivalent ratio (SSC measured at 35 and 10 cmab, respectively) instead averaged \sim 100% (± 10%) throughout the experiment, indicating no substantial vertical structure in the SSCs across these elevations.

DISCUSSION

Despite advances in understanding the interactions between hydrodynamic and sediment transport processes in submerged



canopies in laboratory (physical modeling) studies, there are still limited measurements of the coupled hydrodynamic and sediment transport processes in natural seagrass meadows exposed to a range of met-ocean conditions and with natural sediments. In this study, we present new quantitative insight into how sediment resuspension is affected by the submerged canopy formed by a seagrass meadow. The study site at Garden Island, with abundant seagrass meadows, was largely sheltered from swell but experienced substantial variability in locally generated wind waves due to the strong sea breeze cycle, making it an ideal case study to investigate how different wave and current conditions interact with seagrass canopies to influence near-bed flows and sediment transport. On a weak sea-breeze day, mean currents at the site were weak; however, on a strong sea-breeze day northward-directed mean currents of order 0.1 m/s were generated, which lagged the wind by 1-2 h. Local wind-wave generation also varied in response to the strength of the sea breeze, yet wave heights were influenced by the wind direction that controlled the fetch within Cockburn Sound. In Western Australia, the predominant sea-breeze system generates southerly winds that are more shore-parallel than perpendicular (Pattiaratchi et al., 1997), which determined the fetch available for wave generation. Easterly winds, although weaker, generated larger wind waves when compared to stronger southerly winds due to larger available fetch for waves reaching the site to develop. The wave spectra also varied throughout the day: during the morning, when the sea-breeze was absent or weak, a greater fraction of wave energy occurred within the low frequencies due to some transmission of swell into Cockburn Sound. With the sea-breeze onset, in exchange, higher frequency (sea) waves became dominant.

The seagrass meadow had a substantial influence on attenuating near-bed flow velocities from free-stream (above canopy) velocities. The mean currents within the canopy were substantially more attenuated (average attenuation parameter $\alpha_c = 0.35$) relative to the oscillatory flows generated by the waves (average attenuation parameter $\alpha_w = 0.83$), consistent with both theoretical predictions (e.g., Lowe et al., 2005) and from the limited field studies of flow attenuation in seagrass canopies (Hansen and Reidenbach, 2012). Due to this near-bed flow attenuation, our results reveal the importance of estimating bed shear velocities from measurements taken directly inside the canopy, to provide more robust predictions of sediment resuspension and transport processes within seagrass meadows. Due to the flow reduction inside canopies, our results showed that the bed shear stresses $(u_{*m,bed} \text{ and } u_{*max,bed})$ that exert forces on the underlying seabed were greatly reduced from



those at the top of the canopy $(u_{*m,rough} \text{ and } u_{*max,rough})$. Based on the sediment grain size properties at the site (i.e., D_{50}), a critical shear velocity of approximately 1.2 cm/s was expected to represent the threshold for sediment resuspension at the site. The maximum wave-driven shear velocity associated with the canopy roughness ($u_{*max, rough}$) ranged between 0.3 and 2 cm/s, reaching values repeatedly above this critical threshold; yet no significant variation was observed in SSC for measurements within the seagrass meadow. However, when accounting for the reduction of the bed shear velocity, $u_{*max,bed}$ was much lower (typically 0.7 cm/s), never exceeding the expected critical threshold and thus more consistent with the observed SSC variation over the canopy (Figure 7). In contrast, for measurements over a nearby bare bed site, the maximum bed shear stresses often reached values > 1 cm/s followed by a 10-fold increase in SSC, thus consistent with the estimated critical threshold being exceeded and triggering sediment resuspension. Previously, by studying sediment transport processes in the presence of coral reef canopies, Pomeroy et al. (2017) found bed shear velocities $u_{*m,bed}$ to be ~25% of $u_{*m,rough}$ and $u_{*max,bed}$ to be ~20% of $u_{*max,rough}$. Over a seagrass meadow, we found higher values of $u_{*m,bed}$ representing ~60% of $u_{*m,rough}$ and $u_{*max,bed}$ to be 84% of $u_{*max,rough}$. Pomeroy et al. (2017) also found that currents were reduced to $\sim 25\%$ of the free-stream velocities (35% in our study, in average), while the wave orbital velocities were attenuated to \sim 75% of the free-stream velocities (83% in our study, in average).

As a consequence, if shear velocities were calculated from measurements taken above a canopy, which would represent the bottom stresses the overlying flow would experience, it would have led to large errors in sediment transport predictions, as predicted from flume experiments (Le Bouteiller and Venditti, 2015). These effects can be seen in both time series (Figure 7) and as a correlation between observed SSCs and $u_{*max,bed}$ (Figure 8). Below the required shear velocity threshold, SSC variations were negligible and uncorrelated with the local hydrodynamics, which can be explained by natural variability in SSC not driven by local sediment resuspension. For values above this threshold, which occurred on several occasions for the bare bed experiencing larger bed shear velocities, the SSC was strongly correlated to $u_{*max,bed}$ and $(R^2 = 0.99$ during the bare bed experiment). This suggests that the range and relative proportions of grain sizes in suspension are explained by the maximum shear stresses induced by the wave orbital velocities measured near the bed. Although some laboratory studies have similarly shown how sediment resuspension can be overestimated when using bottom shear stresses over a canopy, our study is the first one (to our knowledge) to quantify these differences between canopy and bed shear stresses within seagrass meadows using in situ data, and in turn how this controls sediment transport.

Despite the wave velocities not being as attenuated as the currents, which would enable waves to act more uniformly from within the canopy up into the water column, we also observed some vertical attenuation of SSC concentrations above the canopy relative to within the canopy, which was not observed in measurements at the same elevations over the bare bed. This implies that even when waves are strong enough to overcome the resuspension threshold in a canopy environment, elevated SSCs may be constrained to within the canopy. A similar effect was suggested by Gacia and Duarte (2001), who observed that the meadow is responsible for buffering sediment resuspension on a similar wind-driven environment and that near-bed flow reductions by seagrass canopies can enhance particle trapping (Gacia et al., 1999; Gacia and Duarte, 2001). Using tracer particles, Agawin and Duarte (2002) found that 70% of the suspended particles within the meadow can be trapped by seagrasses, reducing the suspended sediment that would otherwise be available for current transport above the meadow. In our study site, we used optical sediment instruments located at different heights inside the meadow. As a result, SSC measured above the canopy showed an average of 77% of the values measured near the bed (~ 10 cmab). This difference was smaller when wave-induced shear velocities were higher. However, in the bare bed experiment, the top and bottom SSC data were virtually the same, with SSCs at the top being on average 97% of near-bed values.

Collectively, our results support previous evidence from laboratory and some limited field studies, indicating that seagrass meadows are highly effective in attenuating current flows, but much less effective at attenuating wave-driven oscillatory flows. Sediment resuspension over seagrass meadows would be overestimated by assuming that the flows measured above a seagrass meadow (including the same relative importance of waves and currents) are representative of those responsible for initiating sediment transport.

CONCLUSION

Seagrass meadows provide a wide range of important ecological functions and are highly sensitive to light and turbidity. Despite recent advances in describing sediment resuspension in idealized physical models of seagrass canopies (i.e., in detailed flume experiments), there is a need for developing quantitative process understanding in real, complex meadows. This study shows that models for predicting sediment resuspension for bare sediment beds inaccurately describe the flow-seagrass-sediment interactions, overestimating resuspension. We conducted a detailed field experiment within a seagrass meadow relating hydrodynamics with sediment size and concentration and we found flow velocities at the bed to be significantly reduced by the canopy by an average factor of 35% for currents and 83% for waves. Near-bed shear stress measured within the canopy were strongly correlated with levels of suspended sediment. Once the sediment is resuspended, elevated SSC levels in seagrass meadows are confined to the fraction of the water column occupied by the canopy, in contrast to the bare bed. Therefore, we conclude that there are multiple mechanisms through which a seagrass canopy can modify the sediment transport processes.

The improved process-based understanding of the flow-seagrass-sediment interaction developed here can help inform research into how seagrass meadows can reduce sediment erosion (including their use for nature-based coastal protection), the conditions for when seagrasses can stabilize a seabed to promote seagrass habitat restoration, as well as act as a trap for suspended sediment to enhance water quality and other critical biogeochemical processes.

DATA AVAILABILITY STATEMENT

The datasets analyzed for this study can be obtained by sending a written request to the corresponding author at nery.conttineto@research.uwa.edu.au.

AUTHOR CONTRIBUTIONS

This study was part of NC's Ph.D. research. AP and RL did the fieldwork design and data collection. NC did the data analysis and writing under the review of the other authors, who are his

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