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# The impact of floods triggered by natural dam breakage on the adaptability of downstream river fish—the 2018 baige outburst flood in the upper reaches of the Yangtze river in China

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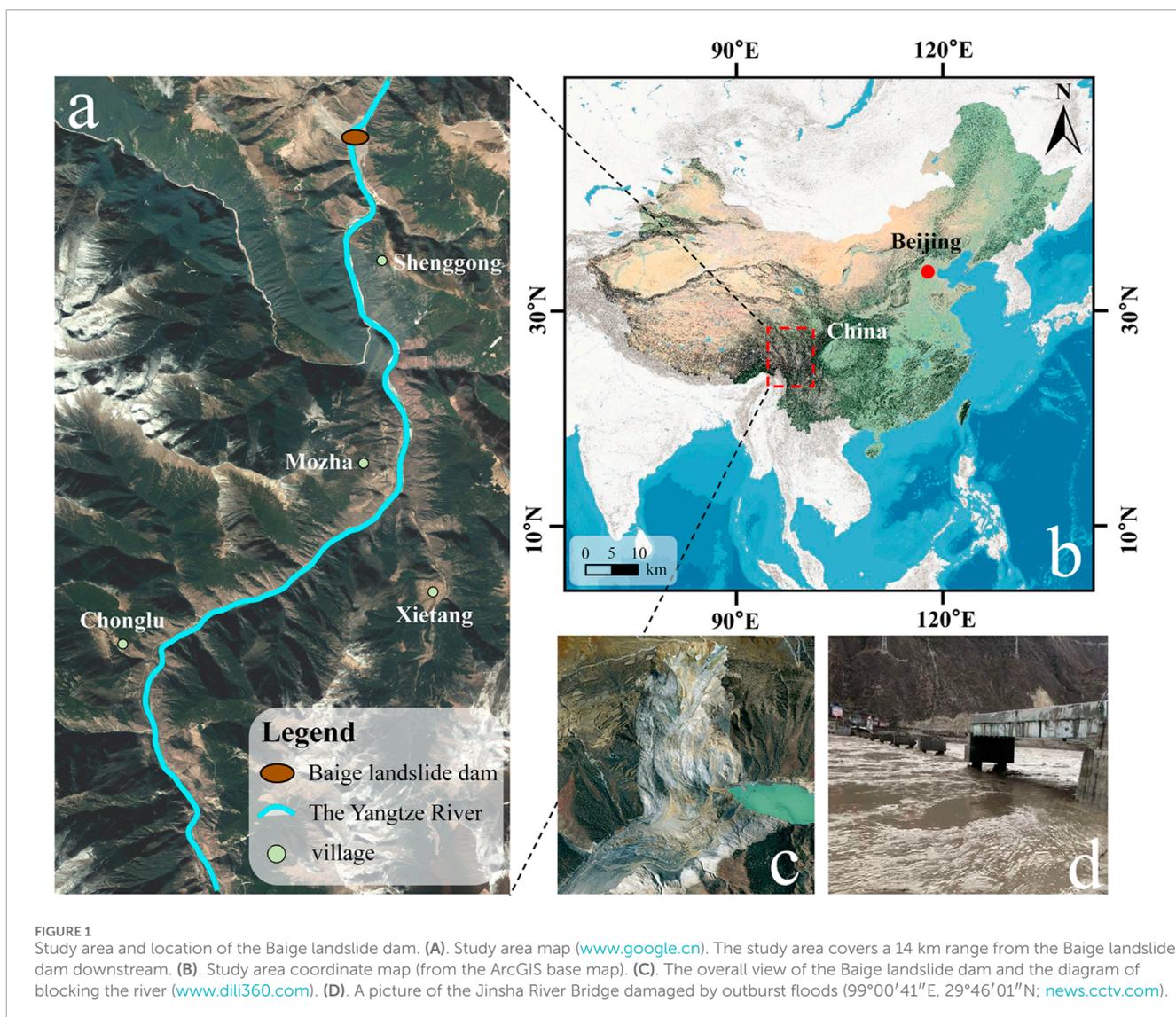
Outburst floods can affect the survival adaptability of fish. Although the survival adaptability of many fish species under low steady-flow conditions has been studied, research on the survival adaptability of fish species under large outburst flood conditions is lacking. This paper takes the 2018 Baige landslide dam as an example. A breach model was developed to calculate the outburst discharge of the landslide dam. The outburst flood hydrograph is simulated with the breach model, which shows that the difference between the peak discharge of the dam break simulation results and the measured data is  $0.13 \times 10^4 \text{ m}^3/\text{s}$ . In addition, the simulated hydrographs are the same as the measured hydrographs. Furthermore, a two-dimensional fish habitat model was used to analyse the adaptability of *Schizothorax* to survival during the breaching process. For the survival adaptability of *Schizothorax*, we observed that as the flow rate increased the weighted usable area (*WUA*) decreased, which indicated a decrease in the adaptability of *Schizothorax* survival. In contrast, as the flow rate decreased and the *WUA* increased, the survival adaptability of *Schizothorax* improved. In addition, the *WUA* of *Schizothorax* changed with the substrate of the riverbed; the smaller the channel suitability index (*CSI*) the greater the *WUA*. This study revealed the impact of outburst floods triggered by landslide dam failure on the survival adaptability of *Schizothorax*, and a method for assessing the impact of outburst floods on fish habitat adaptability is provided.

## KEYWORDS

outburst floods, dam breach Model, River2D Model, *WUA*, fish habitat

## 1 Introduction

An outburst flood triggered by landslide dam failure is a highly destructive and dangerous disaster. The southeastern part of the Qinghai-Tibet Plateau has obvious terrain undulations, which makes it one of the most prone areas to landslide dam failure and flood



disasters in China (Shi et al., 2017). Landslide dams are mainly composed of rock blocks and debris that accumulate in valleys and often block rivers. Statistics indicate that 44%–51% of landslide dams fail within 1 week, 59%–71% fail within 1 month, and 83% fail within 6 months after blockage. Furthermore, floods caused by barrier lake outbursts can propagate tens or even hundreds of miles downstream (Huggel et al., 2002).

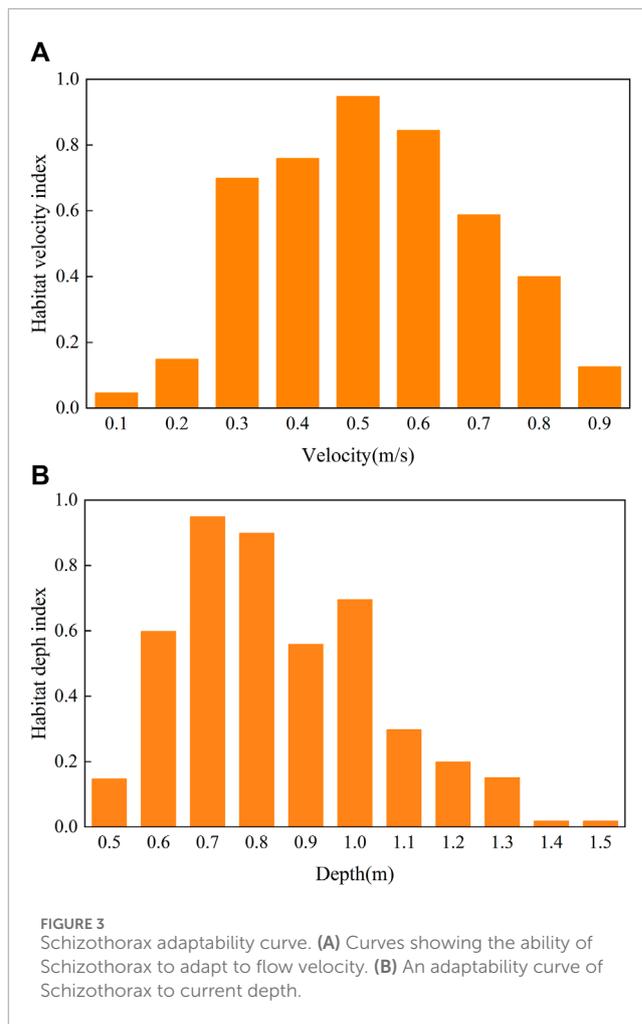
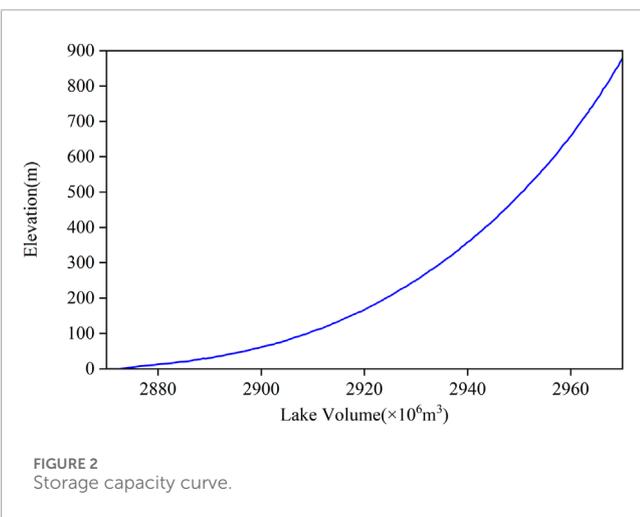
Every year, approximately 2.5 billion people around the world are threatened by floods and approximately 70% of these flood events are related to outburst floods (Herget et al., 2015; Liu et al., 2019; Alfieri et al., 2020; Liu et al., 2023). In the last century, floods, such as the catastrophic floods caused by heavy rains in Hubei Province, China, in 1931 and 1945 (Zong and Chen, 2000; Wang and Plate, 2002) and Bangladesh in 1974 (Hamidifar and Nones, 2023a; Hamidifar and Nones, 2023b), and floods in central China in 1975 (Liu et al., 2019) and in Venezuela in 1999 (Altez and Revet, 2005), have had extremely serious impacts on humans. This is especially true in China as the impacts of climate change and human activities intensify and the frequency and severity of outburst floods are also

increasing (Gao et al., 2021; Liu et al., 2023). For example, outburst floods were triggered by the Sedongpu landslide dam failure in the Brahmaputra River, China, in 2018 (An et al., 2021), and outburst floods were triggered by the Baige landslide dam in the Jinsha River, China, in 2018 (Yang et al., 2022). This trend has caused serious damage to river ecosystems, which serve as important habitats for fish species. The adaptability of fish to flood events has become one of the key factors that determines their survival and reproduction (Pires et al., 2008; Wesner, 2011; Ding, 2005).

The adaptive impacts of flood events on fish are manifold. First, the increase in water level caused by floods not only changes the living environment of fish but also directly affects their physiological functions (Bolland et al., 2015; Hung et al., 2022). Changes in water level can cause hypoxia in fish because the concentration of dissolved oxygen in the water fluctuates as the water level rises and falls. In addition, floods carry large amounts of sediment and pollutants, which may cause harm to the respiratory and digestive systems of fish (Perna and Pearson, 2008). Changes in water pH and temperature caused by flooding may also have an impact on fish

TABLE 1 Data from the Baige landslide dam.

Parameter	Value
Dam top elevation (m)	2,966
Dam width (m)	700
Dam height (m)	96
$\theta_{end}$ (V/H)	1:2.7
$\theta_{up}$ (V/H)	1:1.55
Dam height after failure(m)	35.75
$d_{50}$ (m)	0.005
$n$	0.0557
$C_d$	1.7
$C$ (KPa)	3
$\rho_s$ (kg/m <sup>3</sup> )	1,591
$\rho$ (kg/m <sup>3</sup> )	1,000
Water level (m)	2,952.52
Inflow flow (m <sup>3</sup> /s)	850



growth and reproduction. For example, fluctuations in pH may affect fish bone development and reproductive behaviour, while changes in temperature may affect fish metabolic rates and reproductive cycles (Warren et al., 2010; Solovyev et al., 2018).

Floods not only impact water quality but also lead to damage and loss of fish habitat such as aquatic plants, stones, and trees. However, floods are crucial for the survival and reproduction of fish (Jiménez Segura et al., 2010) and flooding events may also affect fish behaviour. For example, flooding may cause fish to escape or gather in specific areas and thereby affect their normal foraging and breeding behaviours (Boavida et al., 2011; Hogberg and Pegg, 2016).

Water depth, flow velocity, and the composition of streambed material have been identified as major factors affecting fish habitat (Palmer et al., 2007). To date, many scholars have used different methods to study the habitats of fish in different areas. For example, Grüss et al. (2021) utilized the nearshore fish atlas of Alaska and ShoreZone databases to identify *Gadus macrocephalus* in the boreal region of southeastern Alaska as well as Walleye pollock in Prince William Sound. Boudreault et al. (2021) used probability density functions to study Atlantic salmon in the Sainte Marguerite River (Quebec, Canada). Baynes et al. (2023) used generalized linear multiple regression models to study fish species in the Conasauga River in Georgia, United States. Furthermore, the WUA parameter is also usually used to evaluate the habitat of fish. For example, with respect to the WUA parameter, Wang et al. (2023) studied the survival adaptability of fish in mountain areas, and Hung et al. (2022) studied the survival adaptability of fish in the Zengwen River.

This study calculated the outburst flood discharge of the Baige landslide dam and analysed the adaptive habits of fish in the area using WUA parameters and fish habitat models. By studying the changes in fish habitat during the flood process, we explored the impact of floods on fish survival adaptability during the outburst process and changes in fish survival adaptability. Specifically, we

TABLE 2 Substrate index.

Substrate	Fine gravel (0.001–0.002 m)	Medium gravel (0.002–0.006 m)	Great gravel (0.064–0.256 m)	Small stones (<0.02 m)	Large stones (0.02–1 m)	Boulders (>1 m)
Value	0.57	0.83	1.00	0.99	0.95	0.96

selected Schizothorax in the river as the fish species of interest. This paper provides a method and reference for research on fish habitats during outburst floods.

## 2 Materials and methods

### 2.1 Study area

The Jinsha River is located in the southeastern part of the Qinghai-Tibet Plateau. The river has a total length of 3,484 km, the basin area reaches  $5 \times 10^6$  km<sup>2</sup>, and it is one of the tributaries of the Yangtze River Basin (Wang et al., 2015; Xu et al., 2017; Shi et al., 2017). The Qinghai-Tibet Plateau region has obvious terrain relief characteristics. The steep slopes cut by major rivers in mountainous areas aggravate the terrain conditions of landslide dam events. The Qinghai-Tibet Plateau suture zone, which is a strongly tectonic fracture zone, plays a crucial role in the development of landslide dam events (Liang et al., 2018; Liao et al., 2018).

On 10 October 2018, a large-scale landslide occurred on the Jinsha River in the border area between Sichuan Province and Tibet. The landslide blocked the Jinsha River and formed a barrier lake that affected the Sichuan, Yunnan, and Tibet Provinces (Li et al., 2021; Liu et al., 2021).

Due to the landslide, a barrier lake with a volume of approximately  $2.5 \times 10^7$  m<sup>3</sup> was formed. On November 13, an explosive flood occurred with a peak flow of  $3.1 \times 10^4$  m<sup>3</sup>/s (Ouyang et al., 2019; Zhang et al., 2019; Fan et al., 2020; Li et al., 2021; Yang et al., 2022). This catastrophic flood event not only destroyed many bridges (Figure 1D) but also had a catastrophic impact on the lives of more than 20,000 people and a serious impact on fish habitats (An et al., 2021).

This paper examines the 14 km river range downstream of the Baige landslide dam as the study area (Figure 1A). The data used in this paper were obtained from Landsat eight remote sensing images.

### 2.2 Dam breach model

The physics-based simplified dam failure model is currently the most widely used simulation model in disaster risk assessment. This study simulates the dam failure process through a dam failure model that considers the erosion process.

Due to the impounding characteristics of the barrier lake, when the dam starts to breach, the water level and volume of the barrier lake undergo changes influenced by inflows from the upstream basin and outflows from the breach. By combining the area curve of the barrier lake, the water level of the barrier lake can be determined.

According to the principle of water balance, the variation in the volume of the barrier lake is calculated using Eq. 1:

$$\frac{dV}{dt} = A_s \frac{dz_s}{dt} = Q_{in} - Q_k - Q_{spill} \quad (1)$$

where  $t$  is the time (s);  $V$  is the volume of water in the reservoir (m<sup>3</sup>);  $A_s$  is the surface area of the reservoir (m<sup>2</sup>);  $Z_s$  is the elevation of the water level (m);  $Q_{in}$  is the inflow (m<sup>3</sup>/s);  $Q_k$  is the breach flow (m<sup>3</sup>/s); and  $Q_{spill}$  is the spillway flow (m<sup>3</sup>/s). By combining the area curve of the barrier lake, the water level of the barrier lake can be determined, as shown in Eq. 2:

$$A_s = p_1(H_s - H_0)^2 + p_2(H_s - H_0) + p_3 \quad (2)$$

where  $H_s$  is the water level of the lake (m);  $p_1$ ,  $p_2$ , and  $p_3$  are the parameters; and  $H_0$  is the reference height (m).

In this paper, the burst discharge is calculated using Eq. 3 (Coleman et al., 2002):

$$Q = C_d A h^{0.5} \quad (3)$$

In the equation,  $Q$  is the flow rate (m<sup>3</sup>/s);  $A$  is the flow cross-sectional area (m<sup>2</sup>);  $h$  is the head at the flow calculation point; and  $C_d$  is the flow coefficient.

For the erosion calculation, the Meyer-Peter and Muller sediment transport equation was adopted (Meyer-Peter and Müller, 1948) Eqs 4, 5. The equation is as follows:

$$Q_s = \phi [g(\rho_s - \rho) D_{50}^3]^{0.5} / \rho^{0.5} \quad (4)$$

in:

$$\phi = C(\theta - \theta_{cr})^{3/2} \quad (5)$$

In Eq. 5, the  $\theta$  follows Eq. 6 as

$$\theta = \frac{\tau}{(\rho_s - \rho)gD_{50}} \quad (6)$$

In the equation,  $\phi$  is the dimensionless sediment transport rate;  $\theta$  is the dimensionless shear stress;  $\theta_{cr}$  is the dimensionless critical shear stress;  $C$  is the correction coefficient;  $\tau$  is the shear stress (N/m<sup>2</sup>);  $Q_s$  is the sediment transport rate per unit width (m<sup>2</sup>/s);  $D_{50}$  is the median particle size (m);  $\rho$  is the density of water (kg/m<sup>3</sup>);  $\rho_s$  is the density of sediment (kg/m<sup>3</sup>); and  $g$  is the gravitational acceleration (m/s<sup>2</sup>). Among them,  $C=8$  (Li, 2011), and in general, the dimensionless critical shear stress is  $\theta_{cr}=0.047$ .

During the overtop bursting process, vertical and lateral erosion occurs at the breach. These two erosion processes can be calculated

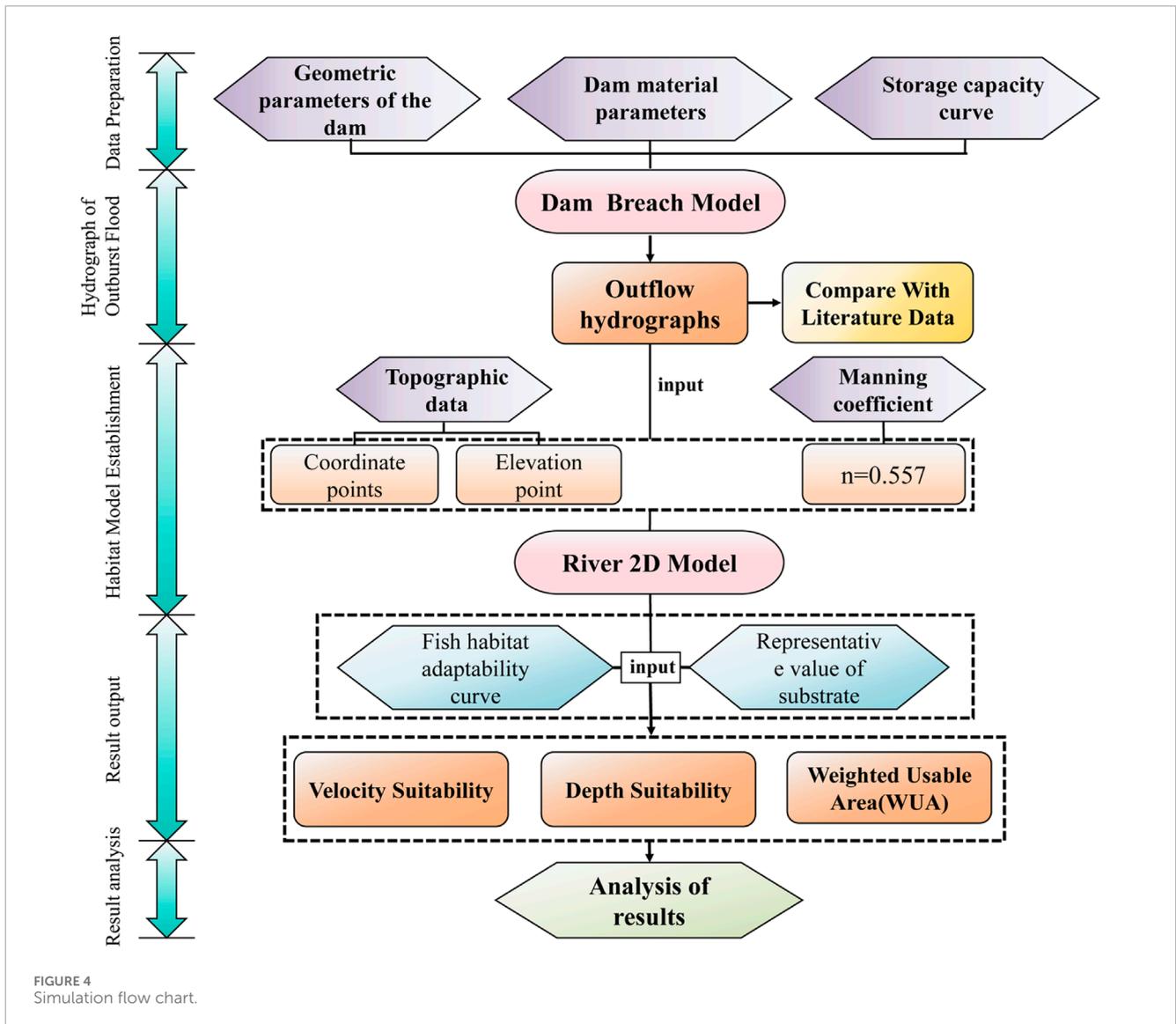


FIGURE 4 Simulation flow chart.

using Eq. 7. According to the conservation of soil mass, the vertical erosion of a breach can be expressed as (Xue et al., 2021):

$$\Delta z = \frac{Q_s \Delta t}{L(1-p)} \tag{7}$$

where  $\Delta z$  is the change in elevation of the breach bottom (m);  $p$  is the porosity (%);  $L$  is the length of the breach channel (m); and  $\Delta t$  is the time change (s).

In addition, the Manning coefficient was calculated with Eq. 8 (Larsen and Lamb, 2016).

$$n = \frac{1}{8} \times \frac{(r_d r_{br} \sigma_{br})^{1/6}}{g^{1/2}} \tag{8}$$

where  $r_d$  and  $r_{br}$  are the hydraulic roughness conversion parameters and  $\sigma_{br}$  is the standard deviation of the bedrock elevation (m).

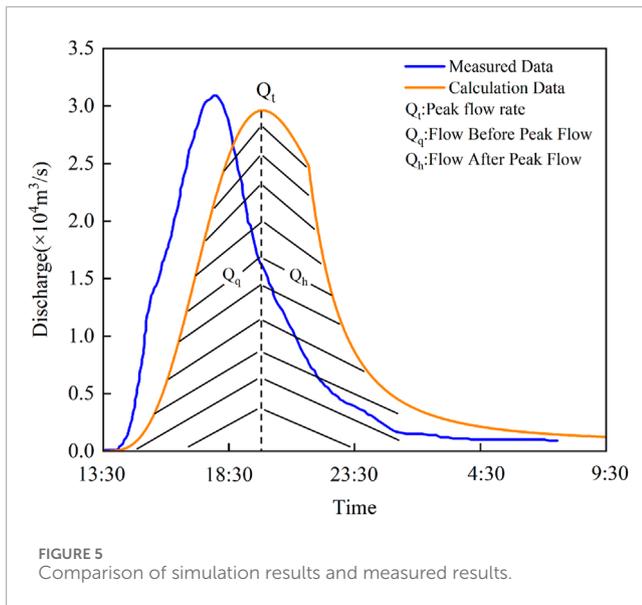
The Manning coefficient we calculated is  $n=0.0557 \text{ m}^{1/3} \text{ s}$  with the above equation. Table 1 shows the data used in this paper.

Figure 2 shows the storage capacity curve of the barrier lake (Liu et al., 2021). All variables are shown in TABLE A1.

## 2.3 Fish habitat model during outburst floods

### 2.3.1 Habitat model establishment

The River2D software was used for the numerical simulation of the habitat in this paper (Boskidis et al., 2018; Baek et al., 2021). River2D is a two-dimensional model of habitat simulation designed specifically for natural streams and rivers. It is a finite element method model that is now used in various studies and works around the world (Lu et al., 2021; Oliveira et al., 2016; Blackburn and She, 2021). The software uses the shallow water equation to analyse the flow hydrodynamic characteristics in the river channel. Among them, the continuity equation and momentum equation constitute governing equations. These equations include the water continuity



equation and the kinetic energy conservation equation in the  $x$  and  $y$  directions (Oliveira et al., 2016; Baek et al., 2021).

River2D requires detailed terrain data for the study area in the form of  $x$ ,  $y$ , and  $z$ , where  $x$  and  $y$  represent geographical coordinates and  $z$  represents terrain elevation. Using DEM data, a large number of  $x$ ,  $y$ , and  $z$  point coordinates were obtained and represent the river terrain and its characteristics (Boskidis et al., 2018).

These points are introduced into the River2D-BDE to create the digital terrain of the river and provide a background for simulating the model. Then, the digital model elevation generated in the above process is introduced into River2D-MESH to generate the research area grid.

The inflow boundary adopts unsteady flow, i.e., the outburst hydrograph, and the outflow boundary adopts the open boundary.

The WUA parameter was used to evaluate the fish habitat. The WUA can be obtained by the Eq. 9:

$$WUA = \sum_{i=1}^n CSF(V_i, D_i, C_i) \times A_i \quad (9)$$

In the equation,  $CSF(V_i, D_i, C_i)$  is the combined suitability value where  $i$  is the number of divided units;  $V_i$  is the flow velocity  $CSF$  of subarea  $i$ ;  $D_i$  is the water depth  $CSF$  of subarea  $i$ ;  $C_i$  is the bottom material or coverage  $CSF$  of subarea  $i$ ; and  $A_i$  is the bottom area of subarea  $i$  ( $m^2$ ). Considering the compensation effects among the factors that constitute the habitat, the Eq. 10 for the suitability value of the habitat combination is:

$$CSF_i = (V_i \times D_i \times C_i)^{1/3} \quad (10)$$

Schizothorax, a common fish that lives near the Baige landslide dam, was selected as the research object. In previous studies, several scholars determined the hydraulic suitability curve (HSC) of Schizothorax through field surveys, as shown in Figure 3 (Hung, H. et al., 2022; Zhou et al., 2019).

### 2.3.2 Substrate adaptability

Fish habitats can also be affected by differences in the substrate. When outburst floods occur, these floods will carry a large amount of sediment and transport the sediment to downstream areas, which causes changes in the river substrate and affects fish habitats. This paper focuses specifically on the adaptability of Schizothorax to survive under different substrate conditions. The applied substrate data are listed in the following table 2:

The research methods and steps are shown in Figure 4, and the process is divided into five total steps. The first stage is the data preparation stage; the prepared data are input into the dam breach model, the dam break model is established, and the outburst flow curve is calculated. The hydrograph curve is combined with the topographic and Manning coefficient data in River2D to establish a Schizothorax habitat model, and finally, the results are exported for analysis.

## 3 Results

### 3.1 Outburst flood analysis

The simulation results of the Baige landslide dam outburst flood is shown in Figure 5. The characteristics of the flow curve indicate that it first rises rapidly to peak flow and then drops to a stable stage. This is consistent with the characteristics of the measured data. In addition, the peak discharge of the simulation results is  $2.96 \times 10^4 \text{ m}^3/\text{s}$ , and the measured discharge is  $3.09 \times 10^4 \text{ m}^3/\text{s}$  (Carling, 2013). There is only a 4.39% difference between the two, which shows that the simulation results are consistent with the measured data. Additionally, the time interval between the measured data and simulated data is 2 h.

The simulation results show that the breach model developed in this paper is suitable for calculating the outburst discharge of landslide dams.

To facilitate the research, we divided the outburst flow into three parts (Figure 5). The flow before peak flow is  $Q_q$ , the flow at peak flow is  $Q_t$ , and the flow after peak flow is  $Q_h$ .

### 3.2 Analysis of the adaptability results

#### 3.2.1 Adaptability of flow speed and flow depth in the river channel

In the simulation, we selected the flow velocity and flow depth adaptability of Schizothorax in the entire study area when  $Q_q=10000 \text{ m}^3/\text{s}$ ,  $Q_t=30000 \text{ m}^3/\text{s}$  and  $Q_h=900 \text{ m}^3/\text{s}$  (Figure 6). As shown in Figures 6A–C, when  $Q_t=30000 \text{ m}^3/\text{s}$ , Schizothorax has the smallest flow velocity adaptability area, which is mainly distributed in a large part of the area and is 0–4.5 km away from the Baige landslide dam. At  $Q_h=900 \text{ m}^3/\text{s}$ , Schizothorax has the largest flow velocity adaptability area and covers the entire river channel. At  $Q_q=10000 \text{ m}^3/\text{s}$ , the distribution characteristics of the flow velocity adaptability are similar to those at  $Q_h=900 \text{ m}^3/\text{s}$ , but the overall area is much smaller than that at  $Q_h=900 \text{ m}^3/\text{s}$ .

The flow depth adaptability is shown in Figures 6D–F. When  $Q_q=10000 \text{ m}^3/\text{s}$ , the flow depth adaptability near the Baige landslide dam is 4.5 km, which is within the range of 8.6

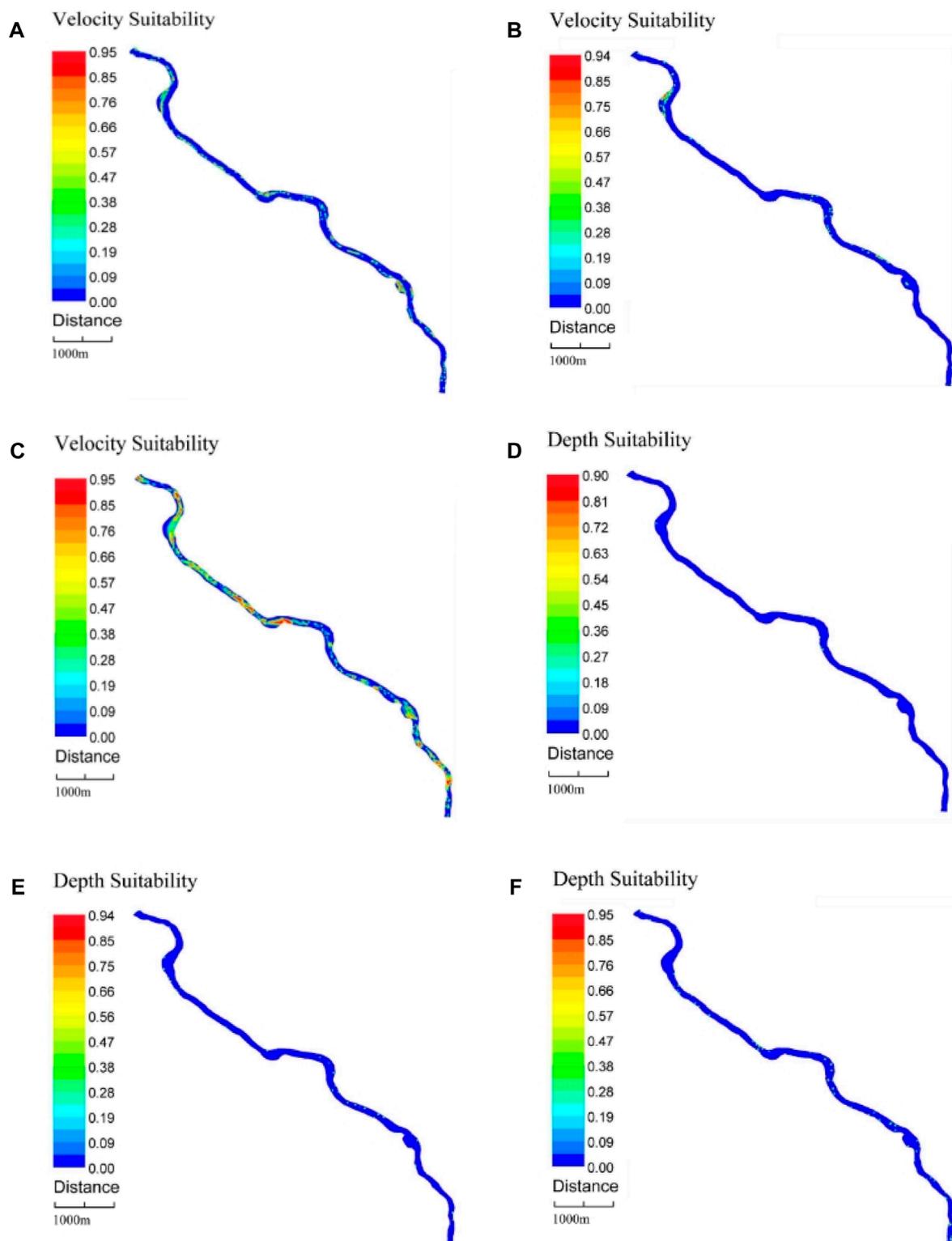
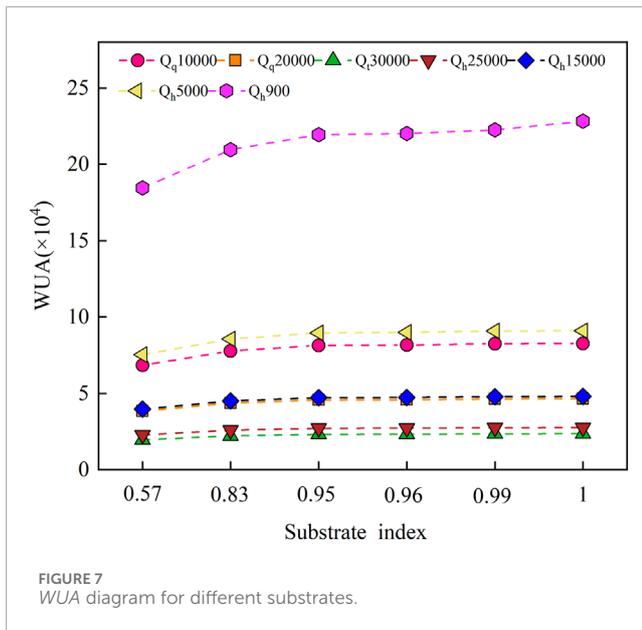


FIGURE 6

Flow velocity and depth adaptability of *Schizothorax* in the river channel. (A) Flow velocity adaptability when  $Q_q = 10000 \text{ m}^3/\text{s}$ . (B) Flow velocity adaptability when  $Q_t = 30000 \text{ m}^3/\text{s}$ . (C) Flow velocity adaptability when  $Q_h = 900 \text{ m}^3/\text{s}$ . (D) Flow depth adaptability when  $Q_q = 10000 \text{ m}^3/\text{s}$ . (E) Flow depth adaptability when  $Q_t = 30000 \text{ m}^3/\text{s}$ . (F) Flow depth adaptability when  $Q_h = 900 \text{ m}^3/\text{s}$ .



km–12.5 km and accounts for a large proportion of the entire study area. At  $Q_t=30000 \text{ m}^3/\text{s}$ , the flow depth adaptability is the lowest across the entire study area. When  $Q_h=900 \text{ m}^3/\text{s}$ , the flow depth has the greatest adaptability and a wide distribution range.

In summary, when an outburst flood occurs, the greater the flow is, the smaller the adaptability zone of the Schizothorax flow velocity and flow depth, whereas the smaller the flow is the larger the adaptability zone. Overall, the adaptive zone of Schizothorax tended to decrease within the study area as the outburst flood flow increased.

### 3.2.2 WUA result analysis

Figure 7 shows that as the substrate adaptability increased, the WUA of Schizothorax under different flow rates also increased. The relationships between the WUA and substrate indices under each flow rate are similar.

Specifically, when  $Q_t=30000 \text{ m}^3/\text{s}$  and  $Q_h=25000 \text{ m}^3/\text{s}$ , the difference in WUA values between the two is small, and the WUA value when  $Q_h=25000 \text{ m}^3/\text{s}$  is greater than the WUA value when  $Q_t=30000 \text{ m}^3/\text{s}$ . Similarly, when  $Q_q=20000 \text{ m}^3/\text{s}$  and  $Q_h=15000 \text{ m}^3/\text{s}$ , the WUA values are almost equal, and the WUA value of  $Q_h=15000 \text{ m}^3/\text{s}$  is greater than the WUA value of  $Q_q=20000 \text{ m}^3/\text{s}$ . When  $Q_q=10000 \text{ m}^3/\text{s}$  and  $Q_h=5,000 \text{ m}^3/\text{s}$ , the difference in WUA values is small, and the WUA value when  $Q_h=5,000 \text{ m}^3/\text{s}$  is greater than the WUA value when  $Q_q=20000 \text{ m}^3/\text{s}$ . Among all traffic flows, when  $Q_h=900 \text{ m}^3/\text{s}$ , the WUA value is the largest. In addition, as the flow rate and substrate adaptability increase, the WUA growth rate under each flow rate also differs. When the substrate adaptability increases from 0.565,065 to 0.827,863, the growth rate is the largest when  $Q_h=900 \text{ m}^3/\text{s}$ , and the growth rate is the smallest when  $Q_t=30000 \text{ m}^3/\text{s}$ . This shows that under low-flow conditions ( $Q_h=900 \text{ m}^3/\text{s}$ ), the changes

in the WUA of Schizothorax to substrate adaptability are more drastic.

Figure 8 shows that regardless of the substrate adaptability of Schizothorax, the entire histogram shows similar characteristics under different flow rates, which shows a trend of changes in the outburst flow curve from large to small and then to large. Specifically, as the flow rate changes, the WUA first increases, then decreases, and then increases on the outburst flow curve. In addition, regardless of the substrate adaptability, when  $Q_t=30000 \text{ m}^3/\text{s}$ , the WUA reaches its minimum value, and when  $Q_h=900 \text{ m}^3/\text{s}$ , the WUA reaches its maximum value. This shows that in the survival adaptability analysis of Schizothorax, discharge had the most significant impact on the WUA.

## 4 Discussion

Many studies have shown that adaptability varies among different fish species in different regions. For example, Hung et al., (2022) used the River2D model to conduct WUA simulations of three different fish species in the Tseng Wen River under ten different constant flows (Figure 9A). Figure 9A shows that when the discharge occurred in the range of 3.38–3,550  $\text{m}^3/\text{s}$ , the WUA of these three fishes tended to increase, while in the range of 3,350–9,490  $\text{m}^3/\text{s}$ , the WUA of Cyprinidae and Gobiidae tended to decrease and the WUA of Balitoridae tended to increase. This shows that the survival adaptability of various fish species changes under different flow conditions, which is attributed to their different flow velocity and flow depth adaptability curves under the same substrate adaptability. Boskidis et al., (2018) also used the River2D model to simulate the habitats of four different fish species in the 134 km channel of the Nestos River basin under five different constant flows. The WUA results are shown in Figure 9B. Similar to the research results of Hsuan-Ju Hung et al. (2022), the WUA of different fish species showed a changing trend under different flow rates, among which the WUA of *Alburnoides strymonicus* decreased when the flow rate increased.

In summary, there are many factors that affect the survival adaptability of fish and the most important is discharge. The same substrate often depends on the flow velocity and flow depth adaptability of the fish itself. Limitations.

This study explored the adaptability of Schizothorax to outburst floods in the upper reaches of the Yangtze River. However, the adaptations of other fish species within this study area have not yet been investigated, which provides ample scope for future research to gain an in-depth understanding of the survival strategies and adaptive mechanisms of different fish species during flooding events. Notably, the topographic changes caused by outburst floods were not considered in this study. However, this study assumed that the topography of the river channel remained unchanged during the flood process. In fact, floods may cause changes in riverbed and bank morphology. In addition, this study only calculated the adaptability of Schizothorax within 14 km downstream of the Baige

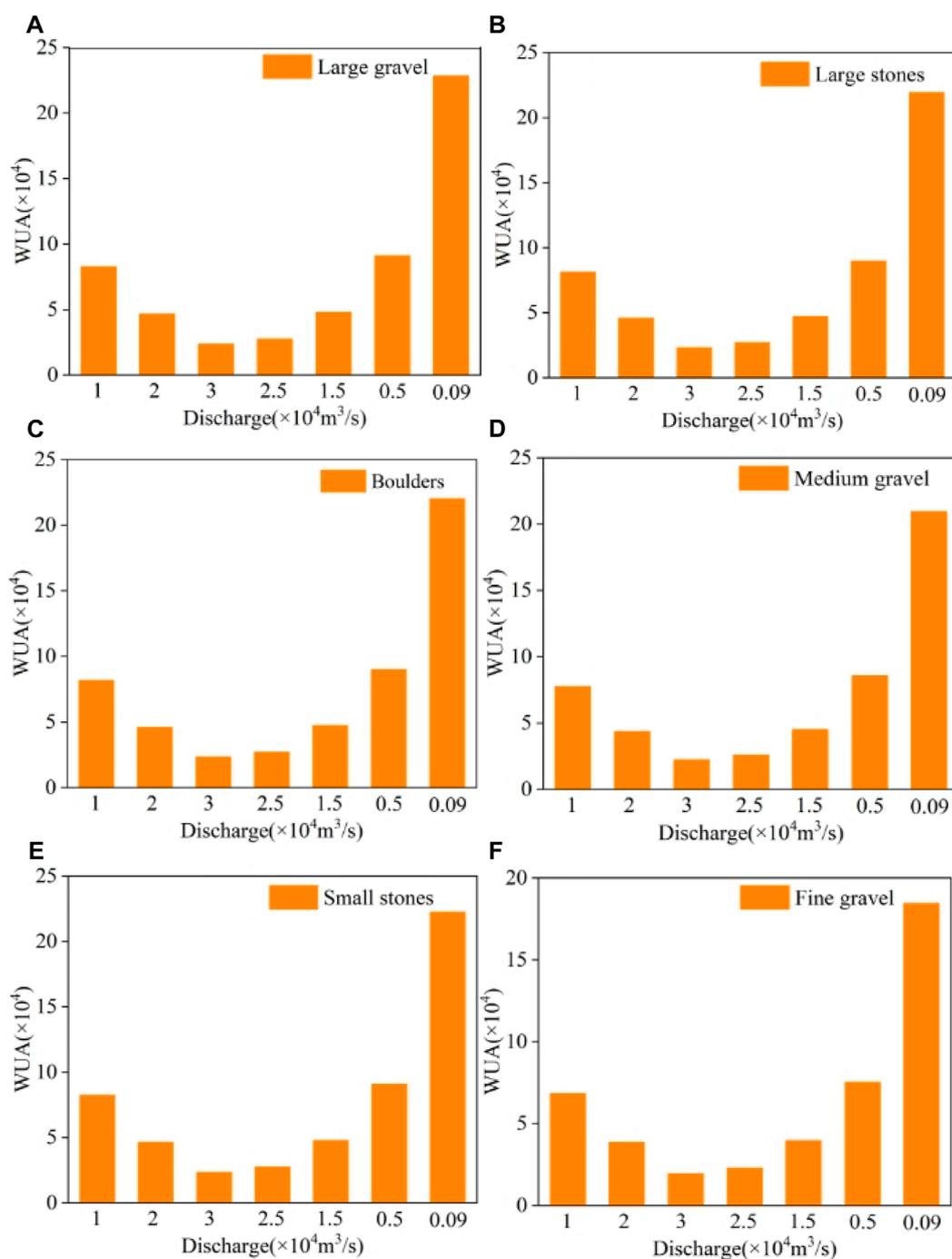


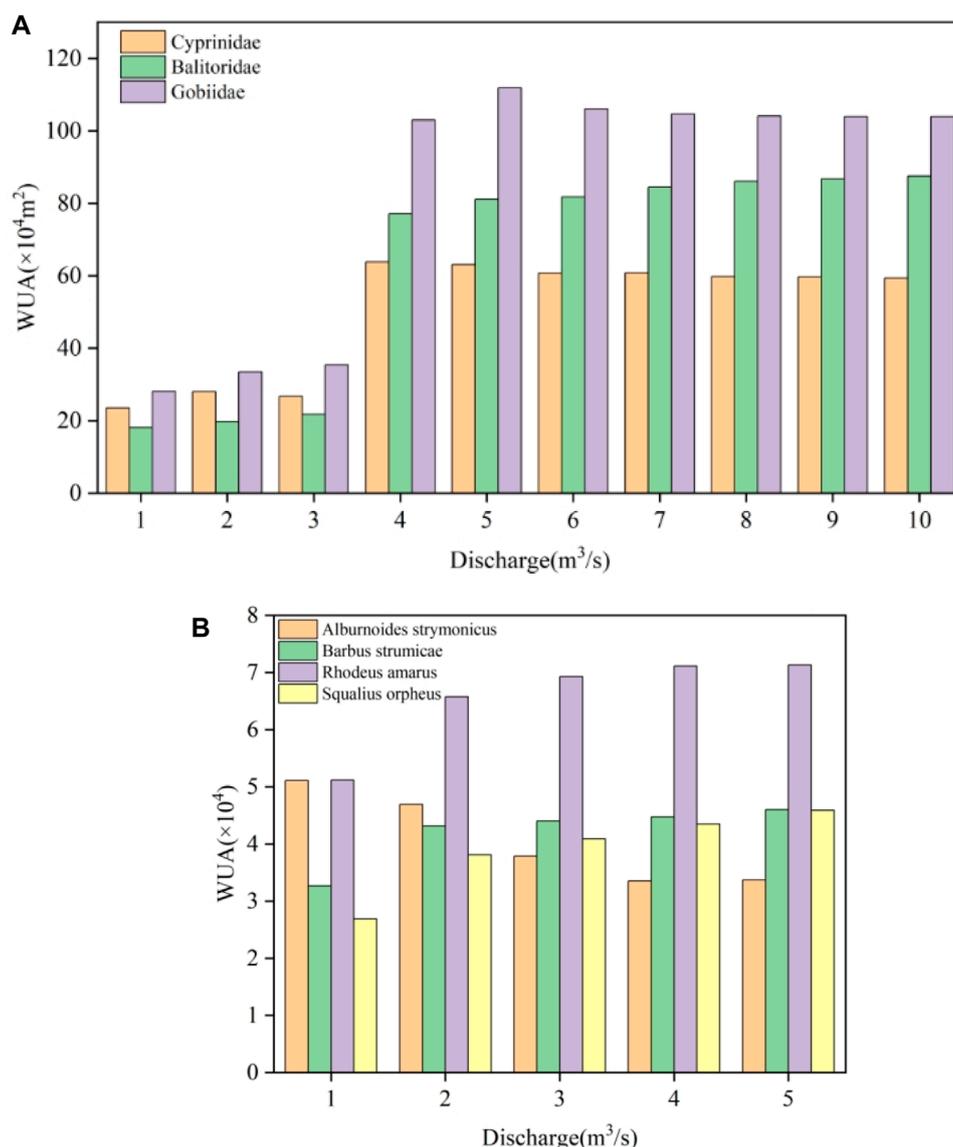
FIGURE 8

WUA graphs under different flow rates. (A) WUA under large gravel. (B) WUA under large stones. (C) WUA under boulders. (D) WUA under medium gravel. (E) WUA under small stones. (F) WUA under fine gravel.

landslide dam. We do not yet know the adaptability of fish in further downstream areas.

Although this study is somewhat limited, it reveals the survival adaptability of *Schizothorax* in the event of outburst floods and under different substrate conditions. This study provides

a reference for research on the survival adaptability of fish under unsteady flow and large flow conditions. However, the lack of real substrate data for the riverbed in the study area limits our conclusions about the actual impact of substrate on *Schizothorax*.



**FIGURE 9** Fish WUA Results. (A) Results from a study by Hung et al. (2022) (B) Results from a study by Boskidis et al. (2018).

## 5 Conclusion

This article provides a detailed simulation of the Baige landslide dam failure in the upper reaches of the Yangtze River and the habitat of Schizothorax. First, the outburst discharge of the dam break model was analysed and compared with the measured flow rate. By comparing measured data and simulation results, we determined that the simulated discharge was consistent with measured data. This is sufficient to demonstrate that the breach model developed in this paper is effective for discharge calculations during dam failure. At the same time, River2D was used to simulate the adaptability of Schizothorax.

To assess the adaptability of Schizothorax, we evaluated two different adaptability parameters under seven different flow rates and six different substrates. The results show that as the outburst discharge increased, the WUA of Schizothorax decreased. The conclusion is the same as that for increasing substrate adaptability. Overall, the method developed in this paper provides a method for studying the impacts of outburst floods on fish habitats.

In future work, expanding the research objects to include more fish types in the study area may be considered. At the same time, the impact of changes in river topography could be considered and more detailed and in-depth research could be conducted to explore the potential impact of these factors on fish adaptability.

## Data availability statement

The original contributions presented in the study are included in the article/Supplementary material, further inquiries can be directed to the corresponding author.

## Author contributions

XX: Conceptualization, Data curation, Formal Analysis, Software, Validation, Writing—original draft, Writing—review and editing. XJ: Conceptualization, Methodology, Validation, Funding acquisition, Writing—original draft, Writing—review and editing, Supervision, Visualization. TW: Software, Validation, Writing—original draft, Writing—review and editing. QJ: Software, Validation, Writing—original draft, Writing—review and editing. XA: Software, Validation, Writing—original draft, Writing—review and editing.

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## Conflict of interest

Author XJ was employed by China Power Construction Group Kunming Survey Design & research Institute Co., Ltd.

The remaining authors declare that the research was conducted in the absence of any commercial or financial relationships that could be construed as a potential conflict of interest.

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## Appendix

TABLE A1 List of symbols.

Variables	Meaning
$V$	Volume of water in the reservoir
$t$	Time(s)
$A_s$	Surface area of the reservoir
$z_s$	Elevation of the water level
$Q_{in}$	Inflow
$Q_k$	Breach flow
$Q_{spill}$	Spillway flow
$H_s$	Water level of the lake
$H_0$	Reference height
$Q$	Flow ( $m^3/s$ )
$A$	Cross-sectional area ( $m^2$ )
$h$	Head of flow calculation point (m)
$C_d$	Flow coefficient
$\phi$	Dimensionless sediment transport
$\theta$	Dimensionless shear stress
$\theta_{cr}$	Dimensionless critical shear stress
$C$	Correction factor
$\tau$	Shear stress ( $N/m^2$ )
$Q_s$	Sediment transport rate per unit width ( $m^2/s$ )
$D_{50}$	Median particle size (m)
$\rho$	Density of water ( $kg/m^3$ )
$\rho_s$	Sediment density ( $kg/m^3$ )
$g$	Gravity acceleration ( $m/s^2$ )
$\Delta z$	Elevation change of breach bottom (m)
$p$	Porosity (%)
$L$	Length of the breach channel (m)
$\Delta t$	Time change (s)
$Z_1$	Dam bottom elevation (m)
$r_d, r_{br}$	Hydraulic roughness conversion parameters
$\sigma_{br}$	Standard deviation of bedrock elevation (m)
$n$	Manning coefficient ( $m^{1/3}S$ )
$H$	Water depth (m)
$V_i$	Flow velocity CSF of subarea $i$
$D_i$	Water depth CSF of subarea $i$
$C_i$	Bottom material or coverage CSF of subarea $i$
$A_i$	Bottom area of subarea $i$ ( $m^2$ )
$Q_q$	Flow before peak flow ( $m^3/s$ )
$Q_t$	Peak flow ( $m^3/s$ )
$Q_h$	Flow after peak flow ( $m^3/s$ )