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RECEIVED 05 July 2023

ACCEPTED 20 May 2024

PUBLISHED 21 June 2024

## CITATION

Bici J (2024), Using borehole radar detecting  
hydraulic fracturing crack in near horizontal  
holes in coal mine.

*Front. Earth Sci.* 12:1253315.

doi: 10.3389/feart.2024.1253315

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# Using borehole radar detecting hydraulic fracturing crack in near horizontal holes in coal mine

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Currently, hydraulic fracturing technology is widely implemented for controlling the surrounding rock and enhancing permeability in low-permeability coal seams. Evaluating the effectiveness of hydraulic fracturing is a critical component of hydraulic fracturing operations. This study addresses the challenges in assessing the location, extension angle, and initial width of fracturing fractures within the current framework of hydraulic fracturing effectiveness evaluation in coal mines. We propose utilizing single-hole reflection borehole radar to evaluate the hydraulic fracturing effect, and through numerical simulation, we analyze the response characteristics of borehole radar when detecting various hydraulic fracturing-induced cracks. Initially, five models representing hydraulic fracturing cracks and two models for non-hydraulic fracturing cracks were established. Subsequently, the responses of borehole radar with central frequencies of 100, 200, and 400 MHz to cracks of identical shapes were analyzed. Additionally, the response characteristics of borehole radar with a 200 MHz central frequency to cracks of varying lengths (1, 2, 3 m), widths (4, 8, 40 cm), and angles (90°, 45°, 15°) were examined. Finally, a comparative analysis was conducted between hydraulic and non-hydraulic fracturing cracks. A branch hole was employed to simulate a hydraulic fracturing crack, allowing for an analysis of the borehole radar's response characteristics in practical scenarios. The findings indicate that borehole radar is a viable tool for assessing hydraulic fracturing effects, providing a theoretical foundation for identifying the position of cracks, evaluating their effectiveness, and determining the regional effectiveness of the hydraulic fracturing crack system.

## KEYWORDS

hydraulic fracturing, borehole radar, crack detection, coal mine, near horizontal hole

## 1 Introduction

Hydraulic fracturing technology, a crucial enhancement method in the oil and gas industry, is utilized to manage the surrounding rock and increase permeability within low-permeability coal seams (Song, 2015; Wang, 2015; Zhao, 2020). Hydraulic fracturing technology uses liquid to transmit pressure and form artificial fractures in formation rocks. Continuous injection of liquid makes the artificial fractures larger, and the liquid brings high-strength solid particles into and fills the fractures. After construction is completed, the liquid is discharged back, and the proppant remains in the fractures, forming a channel with high flow capacity and expanding the seepage area of oil and gas.

The hydraulic fracturing effect evaluation is an important technology in hydraulic fracturing engineering (Shan et al., 2022). The most influential factors on the hydraulic fracturing effect are the expansion form, extension direction and distance of the fracture formed by hydraulic fracturing (Hei et al., 2021; Yuan X, 2019). The fracture-diagnostic technologies include indirect and direct methods. The indirect methods include Well test and History match analysis. The major limitations of this two methods are the following: 1) Model-based assumptions; 2) Requires known values of perm and pressure; 3) Wellbore storage effect; 4) Not suitable for real-time operations. The direct methods include Radioactive Tracers, Caliper log, Temperature log, Production log, Borehole Image Log, Video Camera, Hydraulic Impedance Test, Cross-Dipole Acoustic Log, Deep Shear Wave Imaging, Battery-based Wireless Sensor Network, Battery-less Wireless Sensor Network, Microseismic (Borehole), Microseismic (Surface), Tiltmeter (at Surface), Tiltmeter (Downhole at Offset Well), Tiltmeter at Treatment Well (Downhole), Fiber Optic Cable, Sealed Wellbore Pressure Monitoring, Controlled-Source Electromagnetics, Single hole transient electromagnetic (Zhao et al., 2019; Mo et al., 2020; Zhang et al., 2019; Fan et al., 2016; Duan et al., 2018; Feng et al., 2018). The major limitations for those methods are list in Table 1. In short, the starting position, width, and angle of each single crack in the fracturing fracture cannot be evaluated.

As a geophysical prospecting method with high detection accuracy and large detection range, borehole radar (BHR) has the advantages of continuity, high efficiency and high accuracy. Through the processing and image interpretation of the reflected signals received by the radar host, the purpose of identifying hidden targets can be achieved (Annan et al., 1973; Rossiter et al., 1975). Using BHR to detect cracks in vertical holes on the ground by single hole reflection measurement and cross hole measurement is effective (Liu et al., 2006). Therefore, this paper proposes to use BHR single hole reflection method to detect the hydraulic fracturing cracks in near horizontal holes in coal mine.

This paper uses numerical simulation to analyze the response characteristics of hydraulic fracturing cracks detected by BHR in near horizontal borehole in coal mine. It analyzes the response characteristics of BHR at different crack angles, crack widths and crack lengths. It provides a theoretical basis for using BHR to evaluate the hydraulic fracturing effect in horizontal borehole in coal mine.

## 2 Numerical simulation principle of BHR

At present, there are three kinds of electromagnetic wave numerical simulation methods: ray tracing method, finite difference time domain (FDTD) method and finite element method (Zeng et al., 2010). In this paper the software Gprmax is used. Gprmax is an open source FDTD based ground penetrating radar (GPR) forward simulation software developed by Edinburgh University (Warren et al., 2016). The Gprmax is widely used in the GPR forward simulation.

FDTD divides the simulation space into finite spatial grids (Figure 1). The electromagnetic field of each grid can be determined by six components of  $E_x$ ,  $E_y$ ,  $E_z$ ,  $H_x$ ,  $H_y$  and  $H_z$ , and then the Maxwell equation in time domain can be directly solved by finite difference.

After the initial conditions and boundary conditions of the field are given, the distribution values of the spatial electromagnetic field at each time are obtained in turn, that is, the simulation results of the electromagnetic field in the simulation space area are obtained.

When using Gprmax for FDTD forward modeling of BHR, there are three main parameters to be set: 1) antenna parameters, including excitation source type, antenna spacing, antenna frequency, antenna type, output signal and recording time; 2) Geometric parameters of the model, including model size and mesh size; 3) The physical parameters of the model, that is, the physical parameters of the medium in the model area, including the relative dielectric constant, conductivity, relative permeability, etc., The default boundary condition is the fully absorbed boundary condition (PML) (Zhong, 2008). The details of the rules for setting key parameters are listed on the website <https://docs.gprmax.com/en/latest/input.html>.

## 3 Model design

### 3.1 Model parameters

The analysis is based on the analysis of the hydraulic fracturing in the coal seam, so the non-hydraulic fracturing crack is also considering the crack developed in the coal seam roof. The basic model is a 20 m × 11 m 2D model, in which the coal seam thickness is 5 m, the roof and floor are sandstone, the thickness is 3 m, the borehole diameter is 90 mm, the borehole is 1 m away from the coal seam floor and 4 m from the roof. In practice, workers operate instruments in the roadway, so the designed roadway in the model is 2 m × 5 m. The basic model is shown in Figure 2.

The hydraulic fracturing crack is filled with fracturing fluid or proppant, the non-hydraulic fracturing crack is filled with fissure filler, the roadway is filled with air, and the borehole is filled with fracturing fluid.

The BHR is designed in the form of single hole reflection. There is an offset between the transmitting antenna and the receiving antenna. The offset is 0.5 m. The transmitting antenna is in the front and the receiving antenna is in the rear; The antenna type is dipole antenna, the excitation source is Ricker wavelet, the recorded signal is electric field intensity, the transmitted and received signals of the antenna are Z-direction signals, the recorded and analyzed signals are Z-direction electric field intensity  $E_z$ , the center frequency of the antenna is changed as required, and the grid division is uniform 0.01 m. The boundary condition adopts perfectly matched layer (PML) boundary. According to the model size, the recording time window is 0–100 ns.

According to the parameters requirements of Gprmax, the parameters are shown in Table 2.

TABLE 1 Summary of various fracture-diagnostic technologies with limitations.

Group	Technology	Major limitations
Indirect	Well Test	• Model-based assumptions
		• Requires known values of perm and pressure
		• Wellbore storage effect
		• Not suitable for real-time operations
	History Match Analysis	• Model-based assumptions
		• Requires known values of several reservoir parameters
Direct	Radioactive Tracers	• Depth of investigation ~ 1–2 ft from the well
		• Accuracy dependent on alignment of fracture and well path
		• Not suitable for real-time operations
	Caliper log	• Sensitive to borehole diameter changes
		• Requires cased borehole
		• Not suitable for real-time operations
	Temperature log	• Sensitive to thermal conductivity of different formations
		• Requires multiple passes through the well after hydraulic fracturing treatment
		• Not suitable for real-time operations
	Production log	• Does not provide info about non-producing zones/perfs/clusters
		• Not suitable for real-time operations
	Borehole Image Log	• Requires open-hole borehole
		• Does not provide info about fracture dimensions
		• Not suitable for real-time operations
	Video Camera	• Does not provide info about non-producing zones/perfs/clusters
		• Not suitable for real-time operations
	Hydraulic Impedance Test (HIT)	• Sensitive to tubular diameter changes
		• Not suitable for real-time operations
	Cross-Dipole Acoustic Log	• Cement bond quality
		• Borehole conditions after fracture operations
	Deep Shear Wave Imaging (DSWI)	• Relatively very costly
		• Cement bond quality
		• Borehole conditions after fracture operations
		• Coupling fluid is required in the hole
Battery-based Wireless Sensor Network	• Individual fracture dimensions cannot be determined	
	• Not suitable for real-time operations	

(Continued on the following page)

TABLE 1 (Continued) Summary of various fracture-diagnostic technologies with limitations.

Group	Technology	Major limitations
	Battery-less Wireless Sensor Network	• Signal range limited till the point of their settlement in the fracture
		• Currently at proof-of-concept stage
	Microseismic (Borehole)	• Relatively very costly
		• Requires a large number of vertical wells to monitor a single hydraulic fracturing job
		• Individual fracture dimensions cannot be determined
	Microseismic (Surface)	• Relatively very costly
		• Vertical position of hydraulic fracturing is estimated using a model (P-wave velocity)
		• Individual fracture dimensions cannot be determined
	Tiltmeter (at Surface)	• Individual fracture dimensions cannot be determined
		• Fracture mapping resolution decreases with depth (frac azimuth $\pm 3^\circ$ at 3000ft, and $\pm 10^\circ$ at 10,000 ft)
	Tiltmeter (Downhole at Offset Well)	• Resolution (fracture length and height) decreases as distance of the offset well increases
		• Cannot provide info about fracture growth
	Tiltmeter at Treatment Well (Downhole)	• Fracture length must be estimated indirectly from height and aperture
	Fiber Optic Cable	• Relatively very costly
		• Depth of investigation is 1–2 ft from the fracture event
	Sealed Wellbore Pressure Monitoring (SWPM)	• Requires an offset well that is both non-producing and sealed
		• Depth of investigation is 1–2 ft from the fracture event
	Controlled-Source Electromagnetics (CSEM)	• Relatively very costly
• Model-based assumptions		
• Requires extensive data to build a 3-D forward model used to predict monitoring results		
• Not suitable for real-time operations		
• Individual fracture dimensions cannot be determined		
Single hole transient electromagnetic	• Depth of investigation is 1–2 ft from the fracture event	
	• Individual fracture dimensions cannot be determined	
	• Unable to detect the starting position of fracturing fractures	

### 3.2 The hydraulic fracturing crack model

During the hydraulic fracturing in the coal seam, the crack propagation mechanism is complex, and the crack morphology is also complex (Jiang et al, 2015; 2018; Kang 2015). In order to analyze the response characteristics of the BHR, the model is simplified. When establishing the model of the BHR detecting crack in the coal seam, it is assumed that the hydraulic fracturing crack starts from the borehole wall and extends into the formation, and the crack is considered as a line. In Figure 2, it is marked

as f. Firstly, the response of the BHR antenna at different center frequencies is simulated and analyzed, Then, the parameters of the crack f are changed, we established five hydraulic fracturing crack models:

- 1) Model I: the crack length is 2 m, the angle between the crack and the borehole is  $90^\circ$ , the crack width is 0.04 m, and the center frequencies of the radar antenna are 100, 200, and 400 MHz respectively, as shown in Figure 3.
- 2) Model II: the crack length is 2 m, and the angle between the crack and the borehole is  $90^\circ$ . The width of the crack

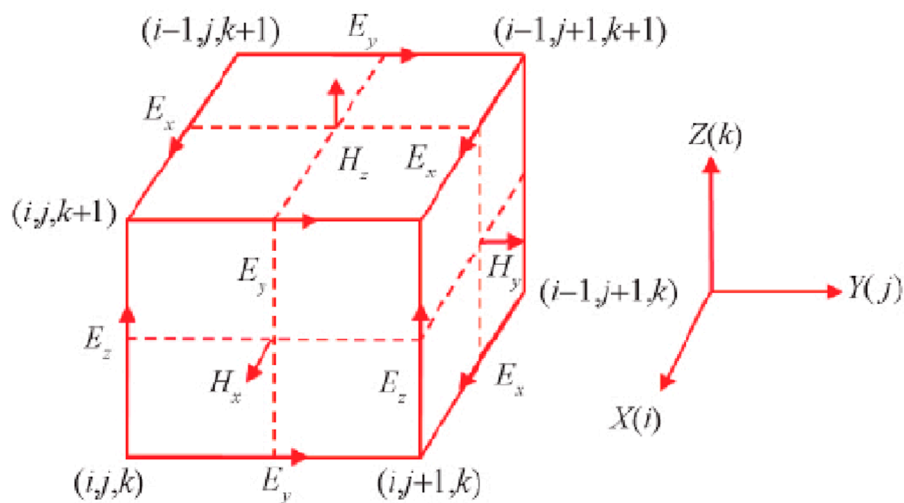


FIGURE 1  
FDTD method difference grid.

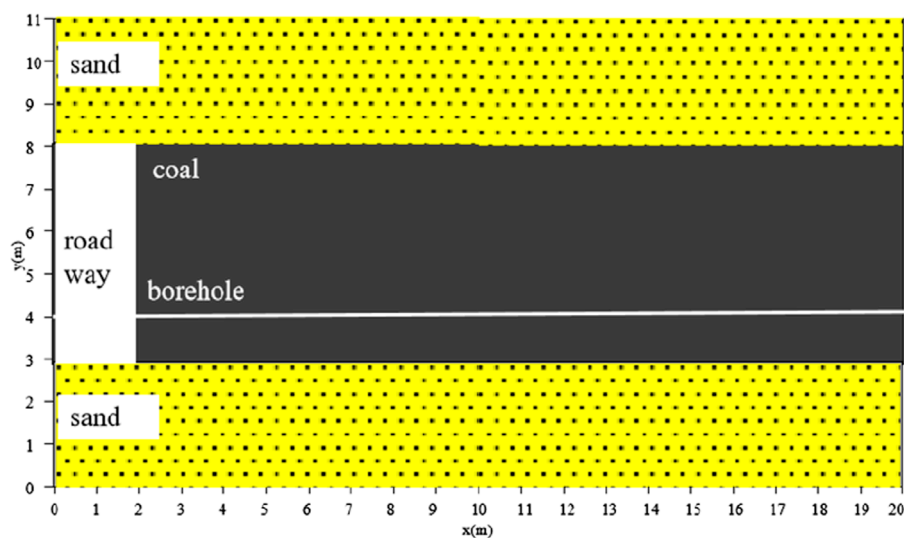


FIGURE 2  
Schematic diagram of basic model.

- f in Figure 3 is changed, which is 0.04, 0.08, and 0.4 m respectively.
- Model III: the crack is 0.04 m wide and 2 m long. The angle of the crack  $f$  in Figure 3 is changed. The angle between the crack and the borehole is  $90^\circ$ ,  $45^\circ$  and  $15^\circ$  respectively. The dotted line  $f_1$  in Fig. 3 corresponds to the crack angle of  $45^\circ$  and the dotted line  $f_2$  corresponds to the crack angle of  $15^\circ$ .
  - Model IV: the crack width is 0.04 m, and the angle between the crack and the borehole is  $90^\circ$ . The longitudinal length of the crack  $f$  in Figure 3 is changed, and the crack length is 1, 2 and 3 m respectively.

- Model V: on the basis of single crack analysis, multiple crack models are designed. The crack width is 0.04 m, the angle between the crack and the borehole is  $90^\circ$ , and the multiple cracks with different lengths are distributed in fishbone shape. As shown in Figure 4, the lengths of  $f_1$ - $f_5$  cracks are 2, 3, 4, 3, and 2 m respectively.

### 3.3 Non-hydraulic fracturing crack model

Coal formation is different from oil formation. Due to mineral changes during deposition, overburden pressure, and mining

TABLE 2 The parameters table.

Parameter type	Parameter name	Value	Parameter name	Value
Antenna parameters	center frequency (MHz)	100, 200, 400	Recording time (ns)	100
	Antenna spacing(m)	0.5	Incentive type	Ricker wavelet
	Antenna type	Dipole antenna	Output signal	Ez
	Antenna direction	Omni-directional Antenna	Antenna length	Point antenna
Geometric parameters	Area	20 m × 11 m	Grid size(m)	0.01
	Coal seam thickness(m)	5	Thickness of top and bottom plate(m)	3
	Road way width(m)	2	Roadway height(m)	5
Physical parameters	Coal seam conductivity (S/m)	0.001	Sand conductivity (S/m)	0.01
	Relative permittivity of coal seam	4	Relative permittivity of sand	6
	Relative permeability of coal seam	1	Relative permeability of sand	1
	Relative dielectric constant of fracturing fluid	30	fracturing fluid conductivity	4
	Relative permeability of fracturing fluid	1	Relative permittivity of fissure filler	10.7
	Relative permeability of fissure filler	1	Conductivity of fissure filler	2.5

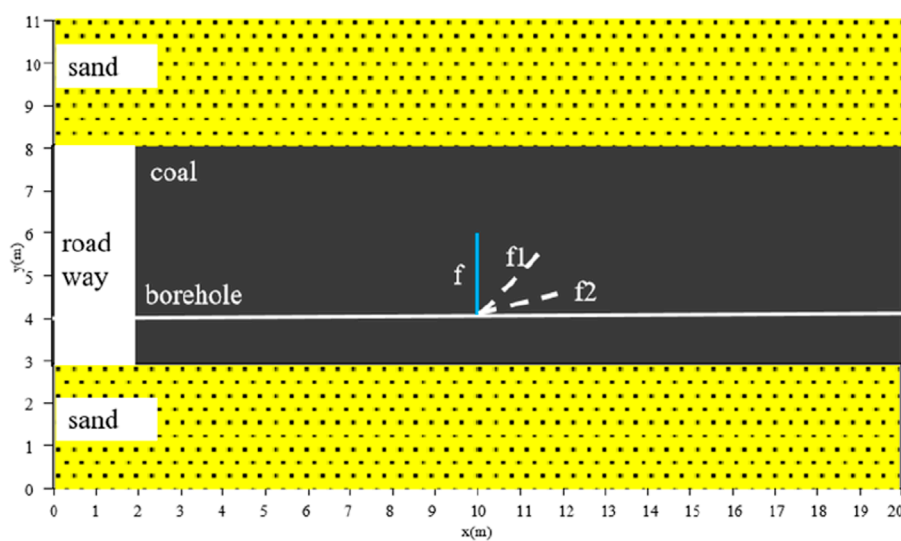


FIGURE 3 Single crack model (Model I-Model IV).

disturbance, there are some cracks formed for other reasons not for hydraulic fracturing (Wang et al, 2015). We call this type crack as non-hydraulic fracturing crack. Two non-hydraulic fracturing crack models are designed:

- 1) Model I: Cracks with different shapes and depths developed in the roof. The lower end of crack  $nf_1$  is 5 m away from the borehole, and the maximum width of crack  $nf_1$  is 0.5 m. The

lower end of crack  $nf_2$  is 3 m away from the borehole, and the maximum width of crack  $nf_2$  is 1 m. The crack model is shown in Figure 5.

- 2) Model II: Cracks penetrating the coal seam are developed in the roof. The cracks shape are irregular, and the maximum width of crack  $nf$  is 0.5 m. The schematic diagram of the model is shown in Figure 6.

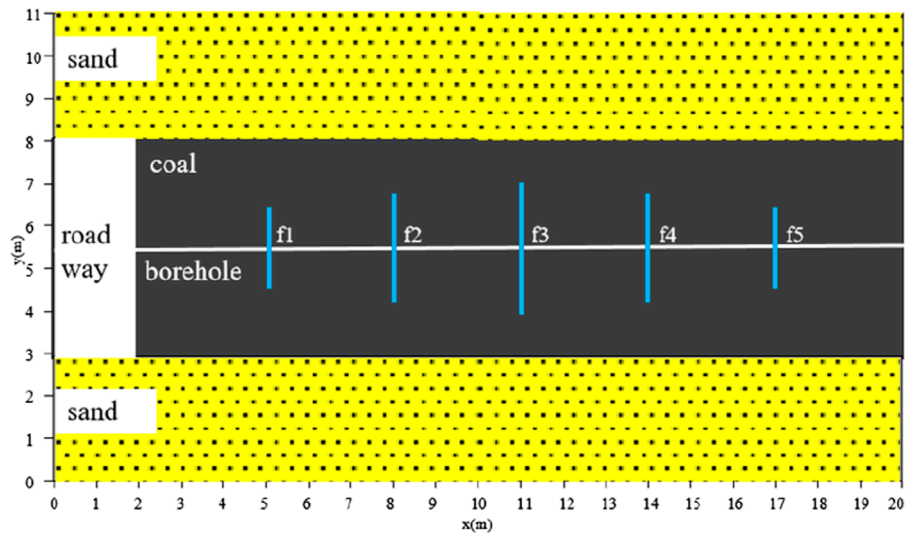


FIGURE 4 Fishbone distribution model (Model V).

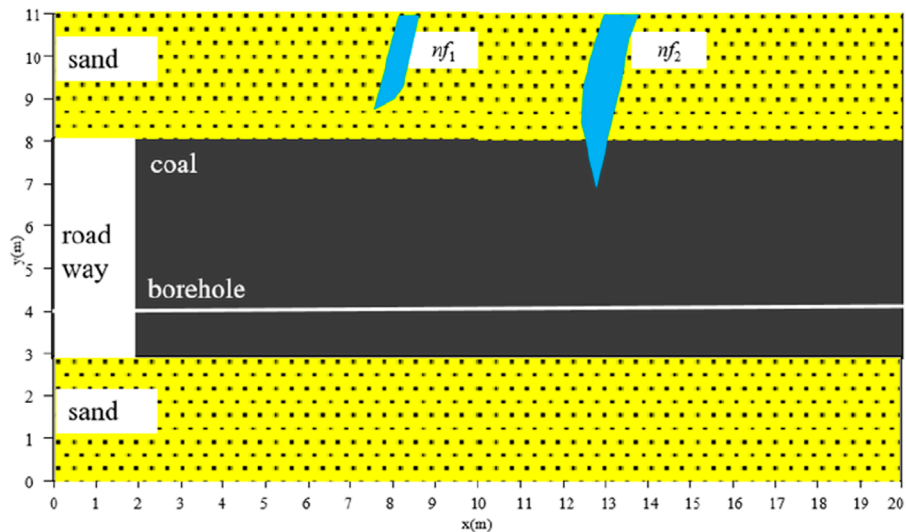


FIGURE 5 Non-hydraulic fracturing crack with different widths and depths models (Model VI).

## 4 Simulation results

Before hydraulic fracturing in the coal seam, the background field is simulated and analyzed to facilitate the analysis of the BHR response characteristics of the cracks. The BHR central frequency is 200 MHz, the BHR response in the coal mine borehole along the coal seam is shown in Figure 7.

In Figure 7, the abscissa represents the depth of the antenna in the borehole, and the ordinate represents the radar response time (the same in the figures below). The phase axis numbered in the figure represents the direct wave (short for “direct wave”), The phase axis numbered represents the floor reflected wave (short for “the floor wave”), the phase axis

numbered represents the roof reflected wave (short for “the roof wave”), the phase axis numbered represents the roadway reflected wave (short for “the road wave”) and the phase axis numbered represents the multiple wave (short for “multiple wave”) respectively.

### 4.1 Hydraulic fracturing crack simulation results

#### 4.1.1 The results of model I

We simulated and analyzed the response with BHR center frequency of 100 MHz, 200 MHz and 400 MHz to detect crack with

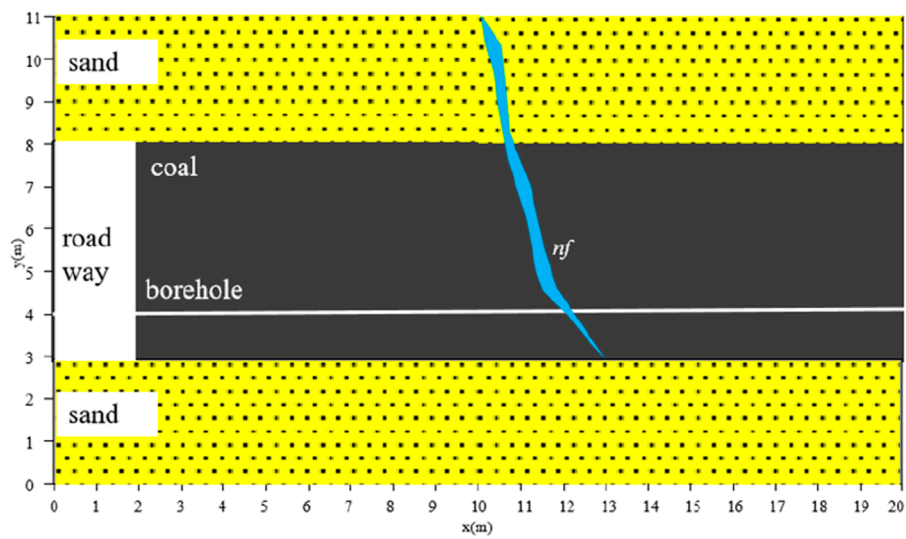


FIGURE 6 Non-hydraulic fracturing crack penetrating coal seam model (Model VII).

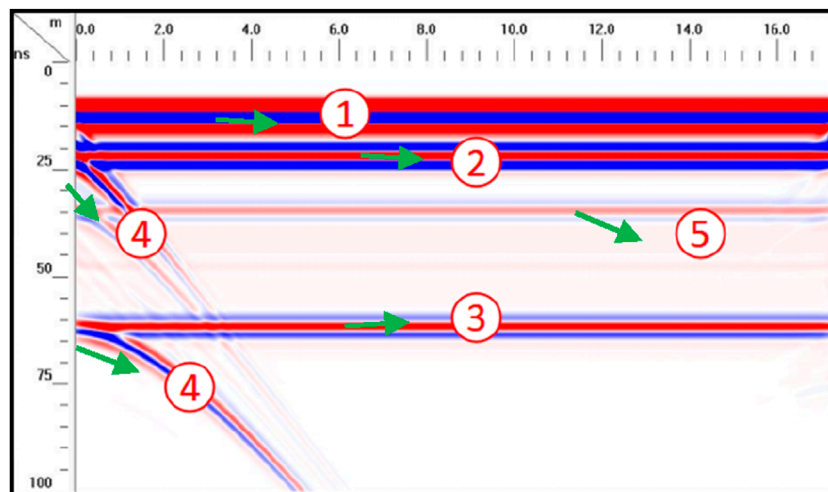


FIGURE 7 The BHR response profile in coal seam (1→direct wave; 2→the floor wave; 3→multiple wave; 4→the road wave; 5→the roof wave).

0.04 m width, 2 m length and 90° crack angle. The simulation results are shown in Figures 8–10.

In Figures 8–10, the reference number ~ phase axis have the same meaning as that in Figure 7. The reference number is the response phase axis of hydraulic fracturing crack (short for “the crack wave”). The upper event is caused by the reflection from the starting of the crack, and the lower event is caused by the reflection from the ending of the crack. On the BHR response time profiles at three frequencies, the phase axis characteristics of hydraulic fracturing crack are obvious,

the crack reflection phase axis in 100 MHz BHR profile are not very clear compared with 200 MHz and 400 MHz. The details of 200 MHz BHR are the same clear as 400 MHz. But in the same time zone, the 400 MHz image color is relative light than the 200 MHz image, which means that the reflected wave energy in 400 MHz BHR profile is weaker than in 200 MHz BHR profile. Therefore, the BHR frequency is too high or too low, which is unfavorable to detection. It is recommended to use 200 MHz borehole radar to detect hydraulic fracturing cracks.



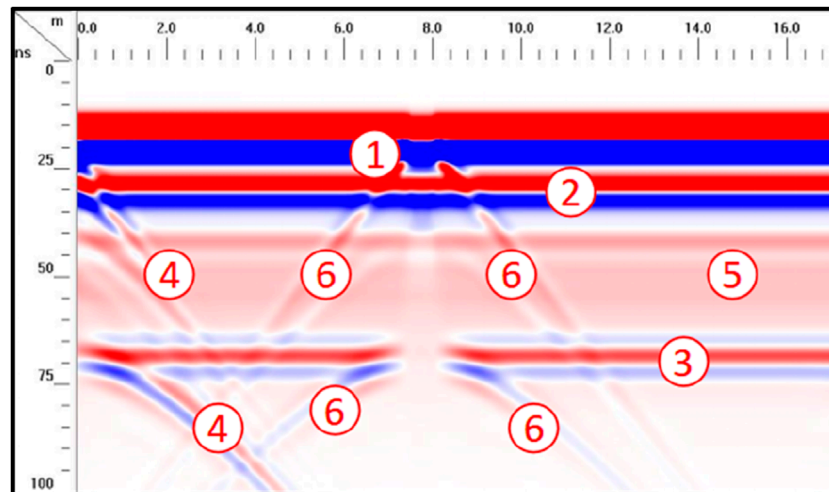


FIGURE 8

The BHR response profile of center frequency 100 MHz (1→direct wave; 2→the floor wave; 3→multiple wave; 4→the road wave; 5→the roof wave; 6→the crack wave).

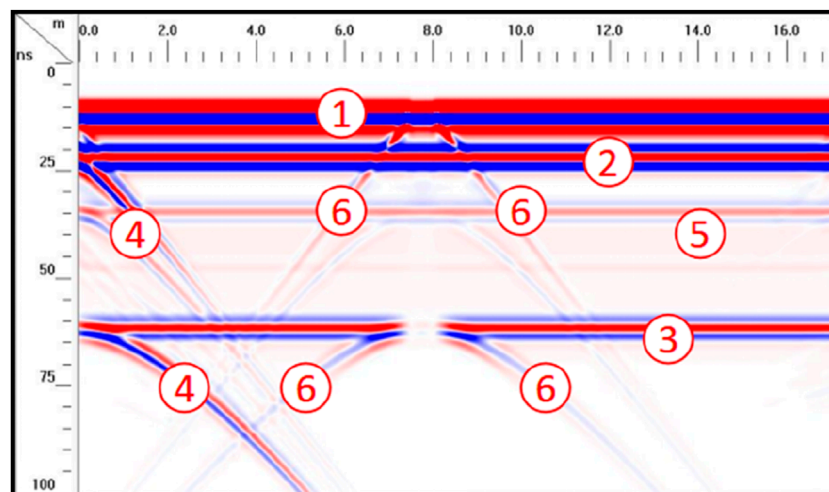


FIGURE 9

The BHR response profile of center frequency 200 MHz (1→direct wave; 2→the floor wave; 3→multiple wave; 4→the road wave; 5→the roof wave; 6→the crack wave).

#### 4.1.2 The results of model II

The cracks length are 2 m, the cracks angle are 90°. The cracks widths are 0.04 m, 0.08 and 0.4 m respectively. According to the previous studies, the 200 MHz BHR is selected. The simulation results are shown in Figures 11, 12 (the response of the model results with the crack width of 0.04 m is shown in Figure 9).

In Figures 11, Figures 12, the phase axis numbered ~phase axis has the same meaning as that in Figure 8. The main difference in Figure 8\11\12 is that the direct wave phase axis and the

interruption width of roof reflection phase axis are different. The width of crack has little influence on the transverse width of the crack phase axis. According to Guo's research, the wider the crack, the stronger the amplitude (Guo S L et al, 2016). So we can use Guo's research to calculate the crack width, the formula is:

$$y = 0.0535x^2 + 0.4648x + 0.8383$$

Where y is the max amplitude intensity, x is the crack width.

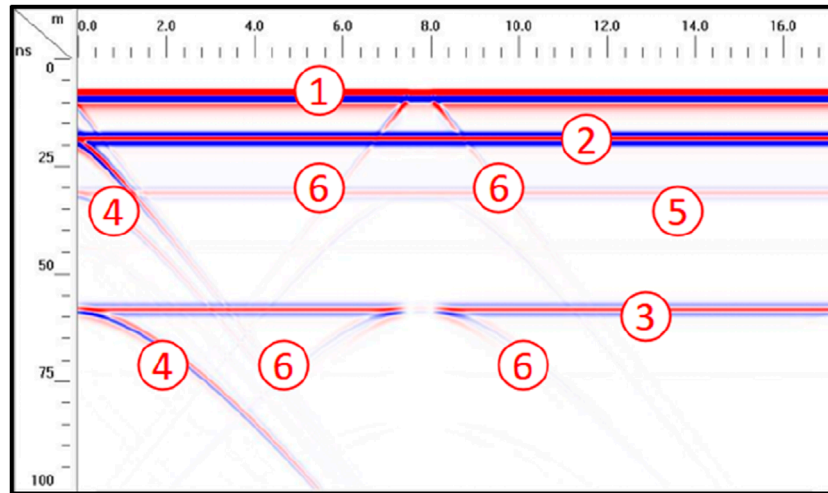


FIGURE 10  
The BHR response profile of center frequency 400 MHz (1→direct wave; 2→the floor wave; 3→ multiple wave; 4→the road wave; 5→the roof wave; 6→the crack wave).

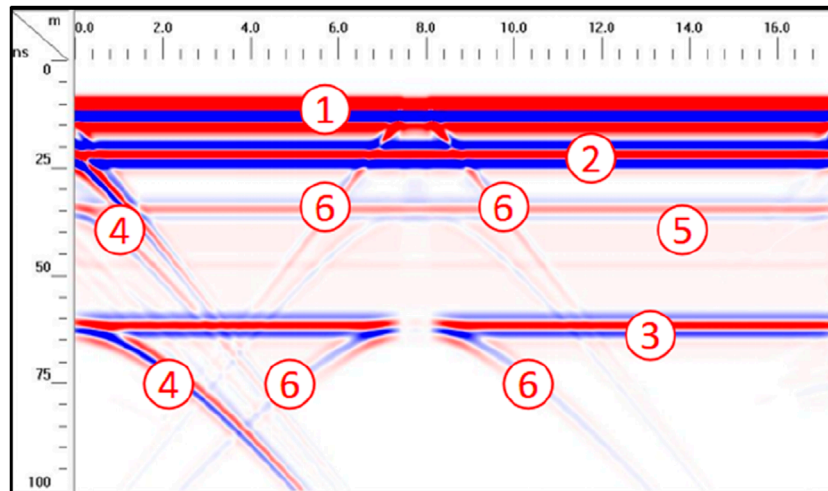


FIGURE 11  
The BHR response profile for 0.08 m width crack (1→direct wave; 2→the floor wave; 3→multiple wave; 4→the road wave; 5→the roof wave; 6→the crack wave).

#### 4.1.3 The results of model III

The cracks angle are  $90^\circ$ . The cracks width are 0.04 m, the cracks length are 1, 2, 3 m respectively. According to the previous studies, the 200 MHz BHR is selected. The simulation results are shown in Figures 9, 13, 14 respectively

In Figures 13, 14, the phase axis numbered ~phase axis have the same meaning as that in Figure 8. By comparing Figures 9, 13, 14, we can find that the difference of reflection phase axes of cracks is mainly in the difference of lateral length of the lower phase axis labeled. The larger the crack length, the longer the phase axis. In practical application, the crack length can be analyzed by judging the lateral length of the phase axis.

#### 4.1.4 The results of model IV

The cracks width are 0.04 m, the cracks length are 2 m, the cracks angle are  $15^\circ$ ,  $45^\circ$  and  $90^\circ$  respectively. According to the previous studies, the 200 MHz BHR is selected. The simulation results are shown in Figures 9, 15, 16 respectively.

In Figures 15, 16, the phase axis numbered ~phase axis has the same meaning as that in Figure 8. By comparing Figures 9, 15, 16, we can find that when the angle between the crack and the borehole is not  $90^\circ$ , the reflection event axis of the crack will not appear in the form of hyperbola. The larger the included angle of the crack, the larger the angle of the reflection event axis, until it appears in the form of symmetric hyperbola at  $90^\circ$ . In practical application, the

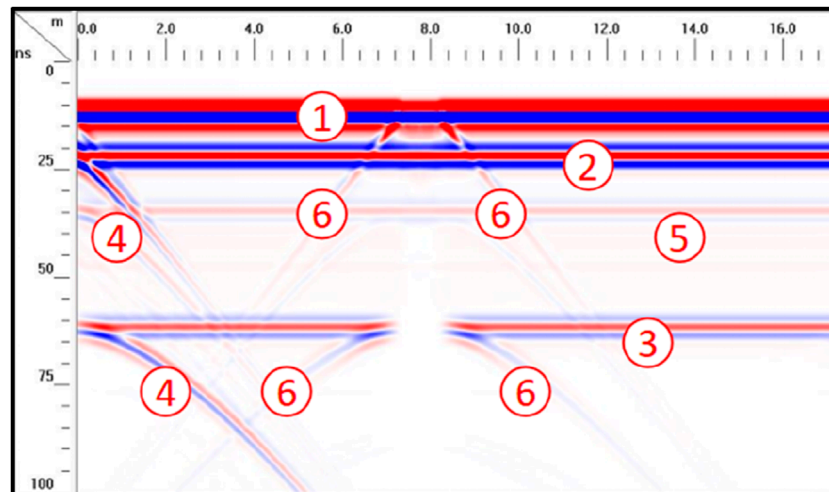


FIGURE 12  
The BHR response profile for 0.4 m width crack (1→direct wave; 2→the floor wave; 3→multiple wave; 4→the road wave; 5→the roof wave; 6→the crack wave).

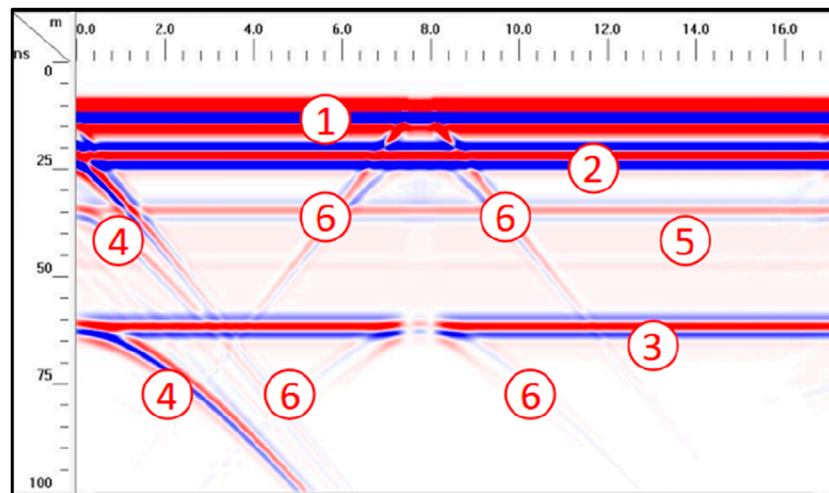


FIGURE 13  
The BHR response profile for 1 m length crack (1→direct wave; 2→the floor wave; 3→multiple wave; 4→the road wave; 5→the roof wave; 6→the crack wave).

crack angle can be analyzed by analyzing the angle between the crack phase axis with the direct phase axis.

#### 4.1.5 The results of model V

The model is described in the model V, the simulated result is shown in Figure 17. In Figure 17, the phase axis numbered ~phase axis has the same meaning as that in Figure 7 and the reference numbers ~ correspond to the response of the cracks f1-f5 in Figure 4. According to Figure 17, when there are multiple fracturing cracks at the same time, multiple cracks can be distinguished on the BHR profile. Five cracks can be clearly identified. In practical application, the BHR can detect multiple hydraulic fracturing cracks at once.

Compared to Figures 11–14, we cannot determine from Figure 17 whether the cracks are developing upwards, downwards, or through the borehole. Which means that the cracks direction can not be recognized.

## 4.2 Simulation results of original cracks

### 4.2.1 The results of model VI

Two primary cracks with different shapes and depths developed in the roof which are shown in Figure 4. According to the previous studies, we also chose 200 MHz BHR

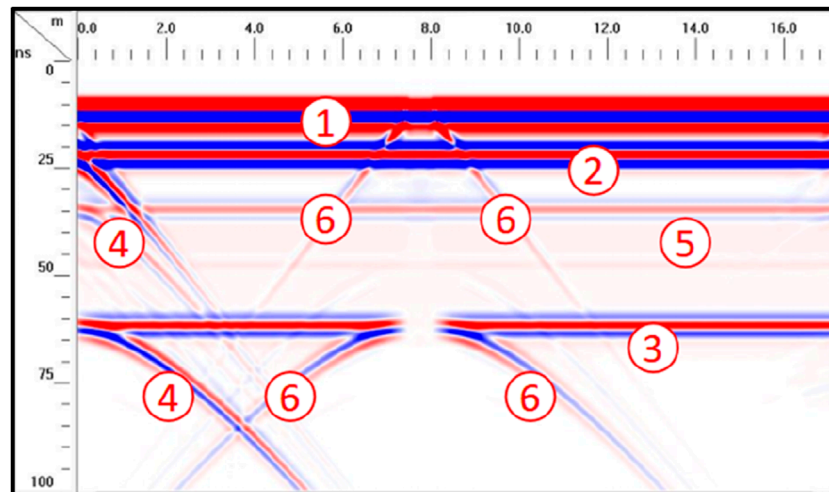


FIGURE 14  
The BHR response profile for 3 m length crack (1→direct wave; 2→the floor wave; 3→multiple wave; 4→the road wave; 5→the roof wave; 6→the crack wave).

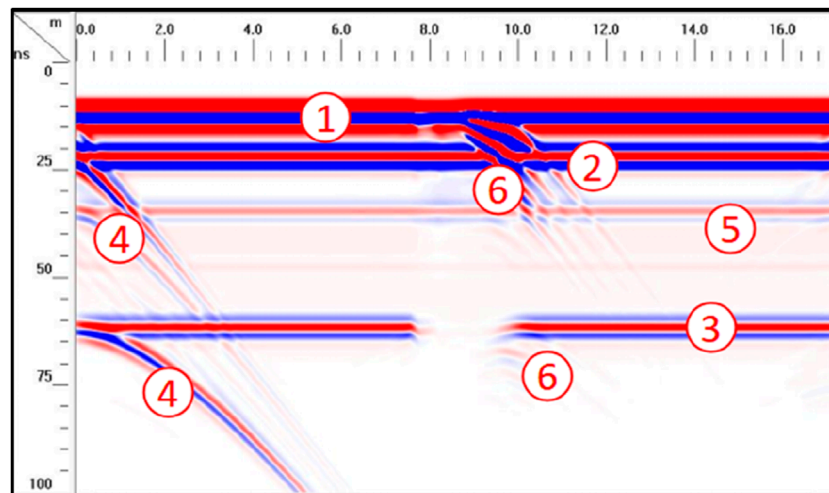


FIGURE 15  
The BHR response profile for 15° angle crack (1→direct wave; 2→the floor wave; 3→multiple wave; 4→the road wave; 5→the roof wave; 6→the crack wave).

to detect the cracks. The simulation results are shown in Figure 18.

In Figure 18, the phase axis numbered  $\sim$ phase axis has the same meaning as that in Figure 7. The phase axis numbered and correspond to the responses of cracks  $nf_1$  and  $nf_2$  in Figure 4 on BHR profile, respectively. According to Figure 18, the distance between the lower end points of  $nf_1$  and  $nf_2$ , and the borehole is different, and the corresponding time between the phase axis numbers and the top end of the in-phase axis is different. Therefore, the distance between the crack and the borehole can be judged according to the position of the in-phase axis of the crack on BHR.

#### 4.2.2 The results of model VII

The crack developed in the roof and penetrating the coal seam is shown in Figure 5. The detection process is the same as before. The simulation results are shown in Figure 19.

In Figure 19, the phase axis numbered  $\sim$ phase axis has the same meaning as that in Figure 7. The phase axis numbered correspond to the response of crack  $nf$  in Figure 5 on BHR profile. Compared with the previous hydraulic fracturing cracks responses shown in Figure 9, the characteristics of this kind of crack on the BHR profile are not different from those of the previous hydraulic fracturing cracks. In the actual hydraulic

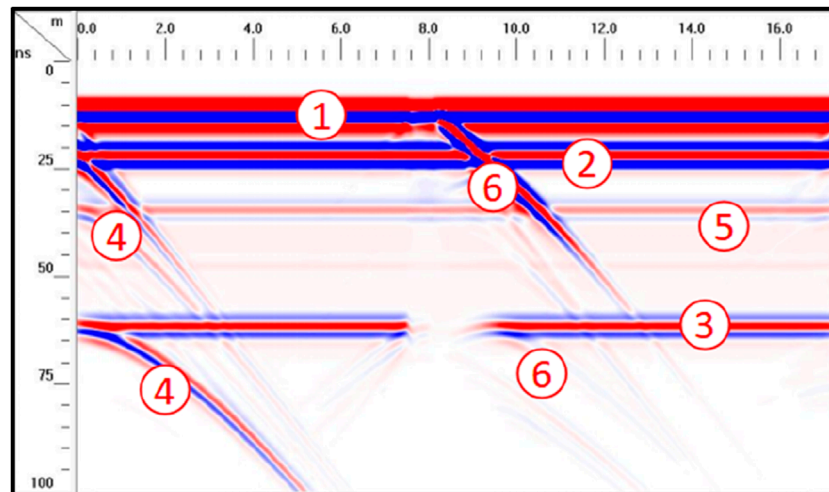


FIGURE 16

The BHR response profile for 45° angle crack (1→direct wave; 2→the floor wave; 3→multiple wave; 4→the road wave; 5→the roof wave; 6→the crack wave).

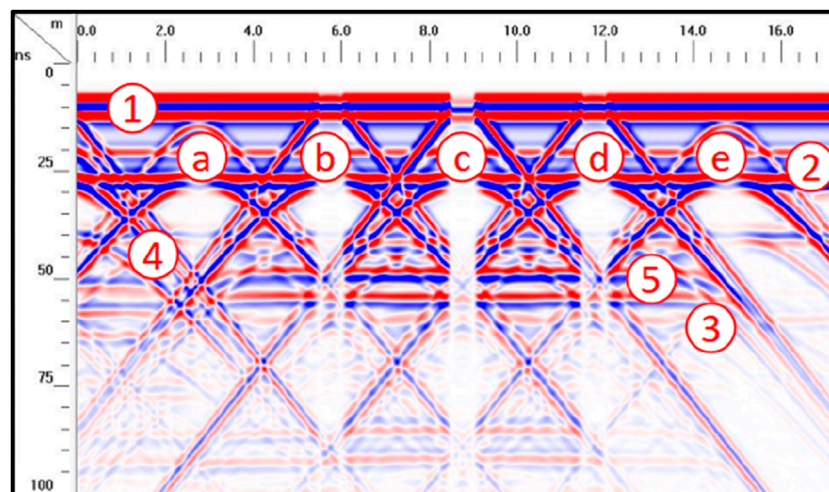


FIGURE 17

BHR response s of fish bone crack (1→direct wave; 2→the floor wave; 3→multiple wave; 4→the road wave; 5→the roof wave; a→the  $f_1$  crack wave; b→the  $f_2$  crack wave; c→the  $f_3$  crack wave; d→the  $f_4$  crack wave; e→the  $f_5$  crack wave).

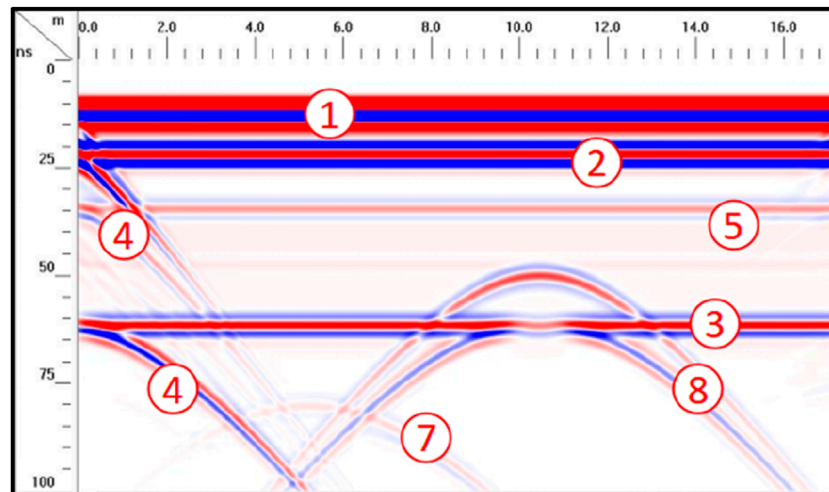
fracturing effect detection, such cracks should also be paid attention for geophysical engineers, so we do not need to distinguish such non-hydraulic fracturing cracks from hydraulic fracturing cracks.

## 5 Application

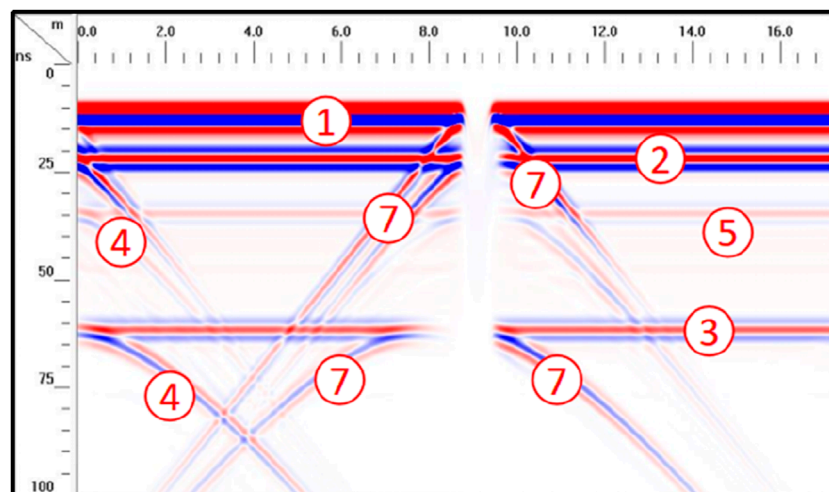
In actual production, we rarely have the opportunity to directly observe the development of cracks in the process of hydraulic

fracturing, however we can leverage the construction of branch holes that occur during the drilling process in coal mines. By treating these branch holes as known hydraulic fracturing cracks, we can use Borehole Radar (BHR) for detection purposes. This approach allows us to assess the effectiveness of BHR in evaluating hydraulic fracturing outcomes.

Before heading of 2,203 working face of a capital construction coal mine in Shanxi Province, directional long boreholes and branch holes were used to detect the geological structures.



**FIGURE 18**  
BHR response of two roof developed cracks (1→direct wave; 2→the floor wave; 3→multiple wave; 4→the road wave; 5→the roof wave; 7→the  $nf_1$  crack wave; 8→the  $nf_2$  crack wave).



**FIGURE 19**  
BHR response of borehole with cracks developed through the coal seam in the roof (1→direct wave; 2→the floor wave; 3→multiple wave; 4→the road wave; 5→the roof wave; 7→the  $nf$  crack wave).

The drilling depth of the main hole was 468 m, and 7 m metal sleeve is placed away from the main hole opening. The borehole diameter is 120 mm, and the 1-1 branch hole is opened at the hole depth of 60 m. The depth of the branch hole is 12 m. The trajectory of the main hole (0–80 m) and the 1-1 branch hole and the formation conditions encountered is shown in Figure 20. The mud circulation drilling is adopted in the drilling process. Therefore, the branch hole detection is used to simulate the characteristics of the cracks detected by the BHR in practice. The BHR center frequency is 200 MHz, and the antenna is sent

into the main hole by hand to measure the depth of 80 m, the instrument photo and parameters are shown in Figure 21. The site construction picture is shown in Figure 22. The original time-domain profile is shown in Figure 23, The data processing flow is as following:

- 1) Time zero correction, to eliminate the influence caused by the delay of the instrument itself;
- 2) AGC processing, to improve the interpretability of BHR images;

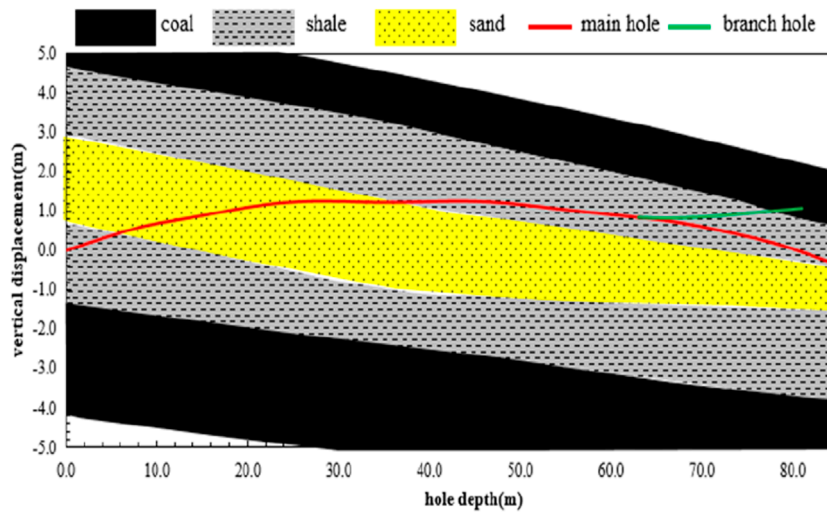


FIGURE 20  
The distribution of holes.

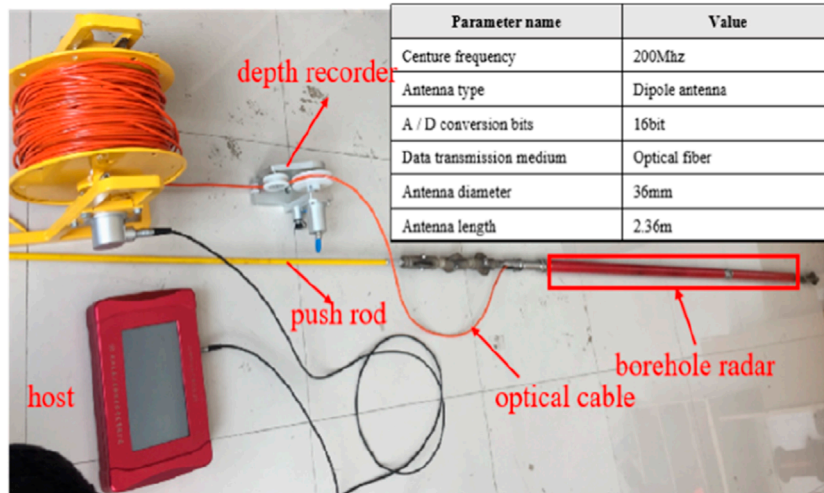


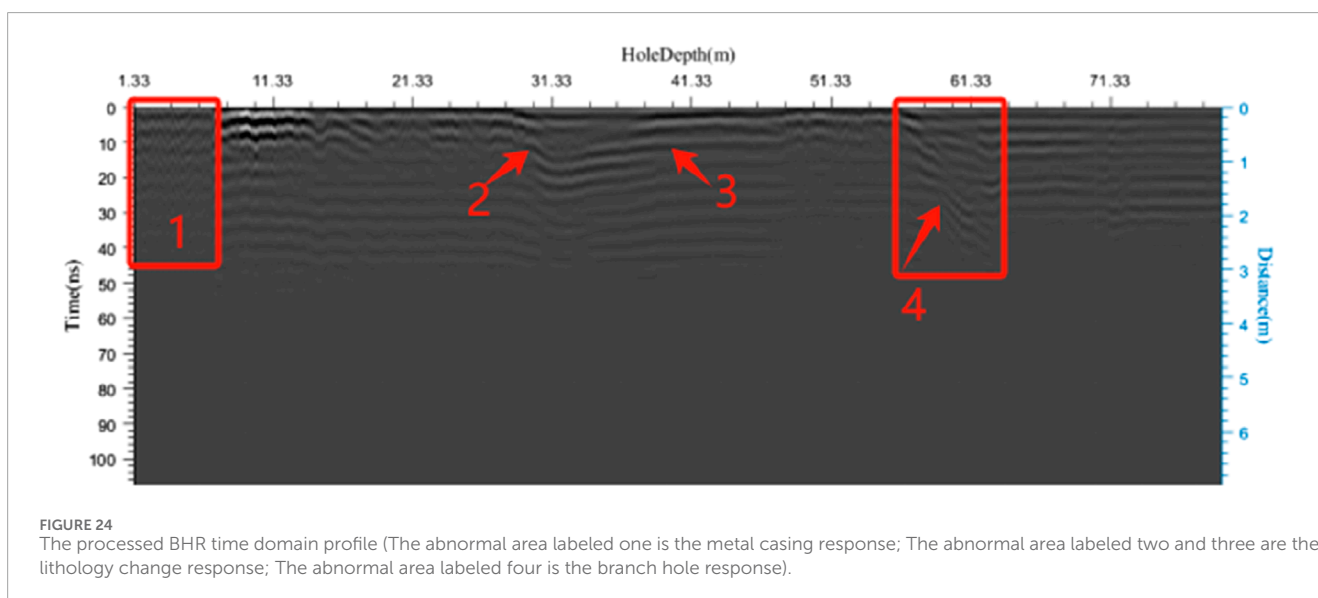
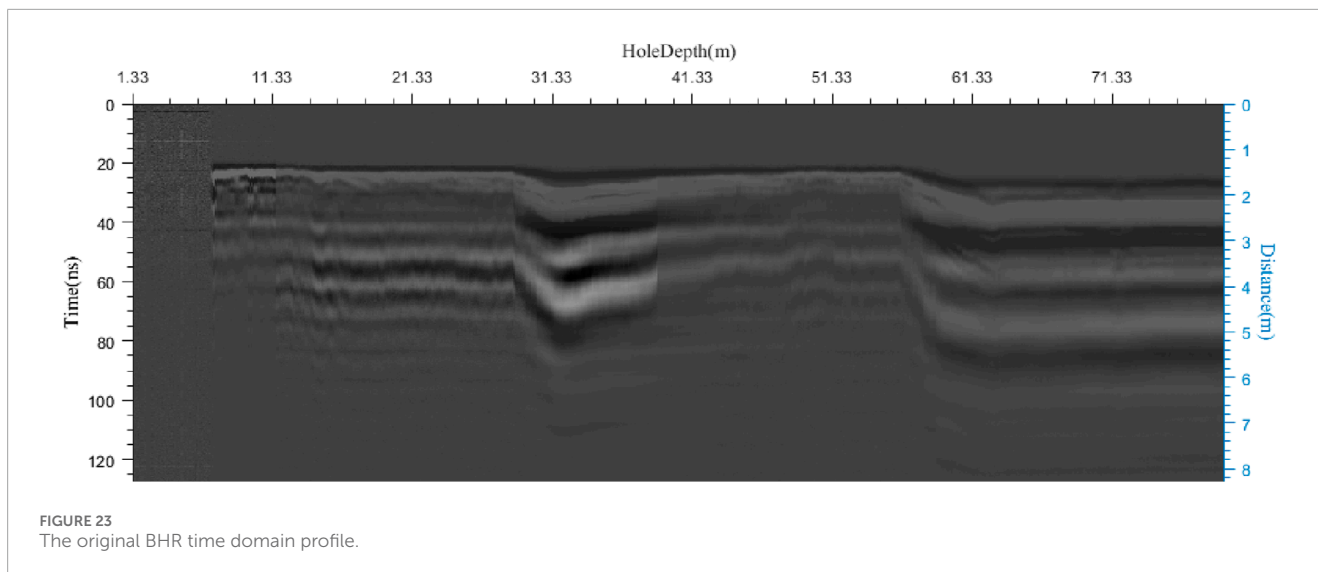
FIGURE 21  
BHR instrument photo.



FIGURE 22  
Site construction photos.

3) Bandpass filtering, to eliminate interference, the bandpass filtering frequency range is from 150 MHz to 400 MHz; The processed the time-domain profile is shown in Figure 24.

In Figure 24, 1) The abnormal area labeled 1 (depth 1.33–7.33 m) is due to the presence of a metal casing as support at the orifice, which shields the signal of the drilling radar inside the metal casing. 2) there are structures at the depth 33 m (labeled 2) and layers at the depth of 40 m (labeled 3) on the BHR profile, which is not conducive to the analysis of the characteristics of branch holes. 3) The characteristics of branch holes at the depth of 60 m are obvious, which is consistent with Figures 14, 15. The length of branch holes cannot be computed on the BHR time-domain profile. Therefore, it is recommended to detect the



original structure of the formation around the hole before hydraulic fracturing. After fracturing, crack detection shall be carried out to compare and analyze the distribution and characteristic of hydraulic fracturing cracks.

## 6 Conclusion

- 1) Single-hole reflection borehole radar (BHR) can be utilized to detect hydraulic fracturing cracks. It is recommended to use a BHR tool with a 200 MHz center frequency for detecting hydraulic fracturing cracks. When employing BHR to detect such cracks, it can ascertain the initial development position of the cracks, determine whether the hydraulic fracturing cracks are perpendicular to the borehole or at an angle other than  $90^\circ$ , and estimate the approximate length of the cracks' development. When multiple cracks are present simultaneously, the response characteristics

of each crack can be clearly distinguished on the BHR time-domain profile. For the original cracks within the formation, the BHR response characteristics differ significantly from those of hydraulic fracturing cracks, allowing for differentiation.

- 2) In practice, when BHR is employed to detect hydraulic fracturing cracks, the geological conditions are complex, with numerous influencing factors, leading to complex BHR response characteristics. It is advisable to first assess the original structure surrounding the borehole before hydraulic fracturing occurs, followed by BHR detection post-fracturing. Through comparison and analysis, the response associated with the cracks can be more clearly delineated.
- 3) For practical applications, single-hole reflection BHR is incapable of calculating the propagation length of hydraulic fracturing cracks. It is suggested that, in the future, cross-hole BHR or array borehole radar



technology be explored to determine the length of crack propagation.

- 4) With omnidirectional drilling radar antennas, determining the direction of crack propagation is not possible. In the future, directional borehole radar antennas may be utilized to ascertain the direction of crack propagation.
- 5) This article presents a novel research direction for the detection of hydraulic fracturing cracks but does not address the response characteristics of borehole radar detection, such as mapping cracks, cross-cracks, fracture permeability, heterogeneity. Further in-depth research in these areas will be necessary in the future

## Data availability statement

The original contributions presented in the study are included in the article/Supplementary Material, further inquiries can be directed to the corresponding author.

## Author contributions

JB: Writing—original draft, Writing—review and editing.

## Funding

The author(s) declare that financial support was received for the research, authorship, and/or publication of this article.

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Thanks for the support by the National Key R and D plan (2018YFC0807804) and Key R&D Plan of Shaanxi Province (2023-YBGY-111).

## Conflict of interest

Author JB was employed by the company China Coal Research Institute Xi'an Science and Industry Group.

The author declares that this study received funding from the Scientific and Technological Innovation projects of CCTEG Xi'an Research Institute (2023XAYJS06) and the General Project of Middling Coal Technology and Industry Group (2022-2-TD-MS005). The funders had the following involvement in the study: provided financial support and essential equipment that facilitated the data collection process for our research article, and participated actively in the study design, data collection and analysis, and decision-making process regarding the publication of the article.

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