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EDITED BY

Erik H. Saenger,
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Eric Larose,
UMR5275 Institut des Sciences de la
Terre (ISTERRE), France
Sebastiano D'Amico,
University of Malta, Malta
Francesco Panzera,
ETH Zurich, Switzerland

*CORRESPONDENCE

Riley Finnegan,
riley.finnegan@utah.edu

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Contribution of anthropogenic vibration sources to crack growth in natural rock arches

Riley Finnegan^{1*}, Jeffrey R. Moore¹ and Paul R. Geimer^{1,2}

¹Department of Geology and Geophysics, University of Utah, Salt Lake City, UT, United States, ²Los Alamos National Laboratory, Los Alamos, NM, United States

Natural arches are culturally valued rock landforms common in sedimentary rocks of the Colorado Plateau and additionally occur broadly around the world. Recent notable collapses of some of these landforms have highlighted the need to better understand the mechanics of their failure. While environmentally driven weathering has been the focus of most previous studies of arch collapse, comparably little attention has been given to anthropogenic vibration sources and how these often slight- to moderate-magnitude shaking events might steadily weaken arches over time. We collected 12–15 months of continuous ambient vibration data from arches and nearby bedrock in both anthropogenically 'noisy' and 'quiet' locations and used these datasets to develop an annual model of arch peak ground velocity based on magnitude-cumulative frequency distributions. Working from these models, we added vibration events of varying magnitude or frequency of occurrence, informed by field data, imitating arch vibration in response to different anthropogenic activities such as helicopter flights or induced earthquakes. We then applied subcritical fracture mechanics principles to predict annual crack growth rates in an idealized arch under these different vibration conditions. Our results demonstrate that in a single year, cracks grow minimally longer (~1%) in 'noisy' environments than in areas not experiencing anthropogenic vibration energy. Few (1+) 30-s moderate-magnitude events (~15 mm/s) or many (>37,000) 30-s low-magnitude events (~2 mm/s) cause markedly increased crack growth. Our approach provides a valuable new framework for assessing the range and frequency of occurrence of vibrations experienced by an arch, and for predicting arch damage. Our results, in turn, yield important new outputs applicable in support of conservation management of these and similar landforms worldwide under exposure to a range of human-induced vibration activity.

KEYWORDS

arches, fractures, vibration, highways, trains, helicopters, earthquakes

1 Introduction

Arches experience gravitational stresses that are often well below typical values of rock strength (in both compression and tension; Moore et al., 2020), yet we observe cracks and partial arch collapses *in-situ* (Figure 1). Notable recent collapses of culturally valued landforms include Malta's Azure Window in 2017 (Satariano and Gauci, 2019), Arch Rock, California, United States, in 2015 (killing one person and severely injuring another; San Francisco Chronical, 2015), the partial collapse of Landscape Arch, Utah, United States, in 1991 (Deseret News, 1991), and the total collapse of Wall Arch, Utah, United States, in 2008 (Willis, 2009). Fractures of all scales permeate rock masses, and new crack growth with accompanying rockfall are often observed prior to landform collapse, driven strongly by changes in moisture (Nara et al., 2012; Voigtländer et al., 2018) and temperature (Atkinson, 1984; Gischig et al., 2011). However, aside from the contribution of climate change on crack growth in rocks (Eppes et al., 2020), few studies have quantitatively analyzed the impact of anthropogenic

activity on structural degradation of rock landforms, specifically rock arches (King, 2001; King and DeMarco, 2003; Finnegan et al., 2021, Finnegan et al., 2022).

With *in-situ* stresses often well below critical rock strength thresholds (Moore et al., 2020), analysis of crack propagation in rock arches requires adopting subcritical fracture mechanics concepts (Eppes and Keanini, 2017). Subcritical cracking does not employ critical equilibrium laws, but rather kinetic laws that involve description of crack geometry and stress levels to assess crack growth rates. Laboratory studies in particular show that rates of subcritical crack growth increase dramatically with increasing subcritical stress intensity factor (e.g., Eppes and Keanini, 2017; Nara et al., 2017), adhering to a power law relationship with an exponent of 60 for sandstones, possibly higher. While some discussion relates to the propagation of cracks beyond a crack-tip immediate damage accumulation zone (Brain et al., 2014), field observations from natural arches in the Colorado Plateau demonstrate that cracks in arches are present and propagating under subcritical stress conditions. These observations allow us to approximate arch



FIGURE1

Example beam-like rock arches, with various observed tensile cracks. Arch spans given in bottom corners. (A) Rainbow Arch, Arches National Park, UT (now collapsed). Scale bars shown for each inset, and dashed box shows location of inset photos. (B) Little Bridge Arch, Moab, UT. (C) Two Bridge, Bryce Canyon National Park, UT. (D) Cobblestone Natural Bridge, Arches National Park, UT.

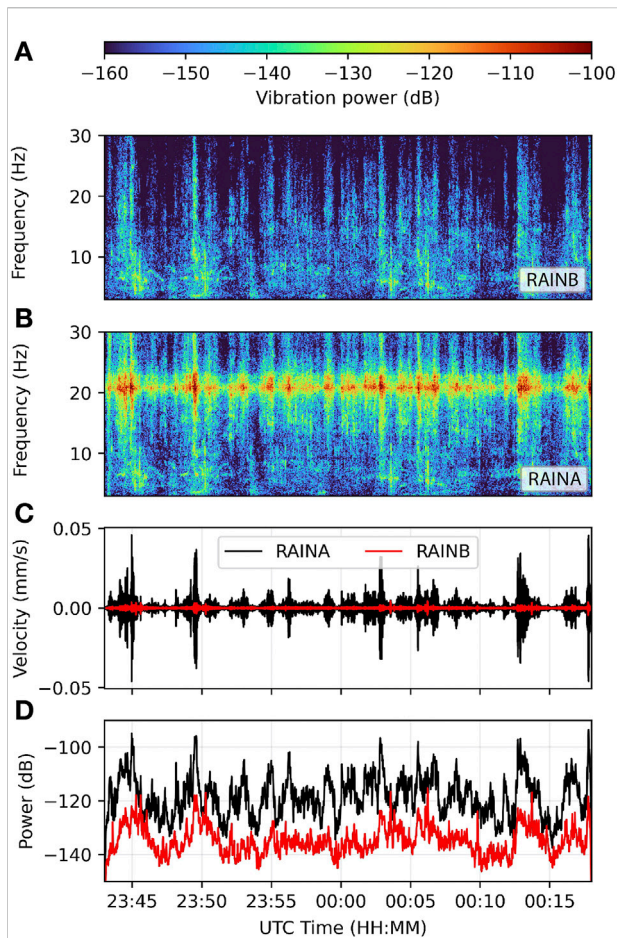


FIGURE 2
Sample of Rainbow Arch (RAINA) and Reference (RAINB) vibration data from 8 to 9 November 2017. (A) Vertical vibration velocity spectrogram of Rainbow Arch bedrock reference during a period of frequent truck passes. (B) Vertical vibration velocity spectrogram from the center-span of Rainbow Arch. (C) Vertical vibration velocity of arch and bedrock reference. (D) Vertical vibration power of arch and bedrock reference. Vibration power in panels (A), (B), and (D) is given in decibel units relative to $1 \text{ m}^2\text{s}^{-2}\text{Hz}^{-1}$.

stresses and crack lengths—e.g., derive stresses from displacements integrated from velocity measurements (Adams et al., 2005)—and use these data to estimate the crack intensity factor (K_I), which is in turn used to estimate the rate of crack growth (e.g., Eppes and Keanini, 2017; Nara et al., 2017).

Road and railway traffic generate ground borne vibration energy across a frequency range that coincides with the natural frequencies of many rock landforms, including arches (Whiffin and Leonard, 1971; King et al., 1985; Hanson et al., 1991; Volpe National Transportation Systems Center, 2014; Díaz et al., 2017; Meng et al., 2021), and is thus capable of exciting resonance and increased amplitude vibration. Our field data indicate that trucks passing on a highway ~175 m from an arch generate prominent spectral energy in the ~10–30 Hz band and are able to excite

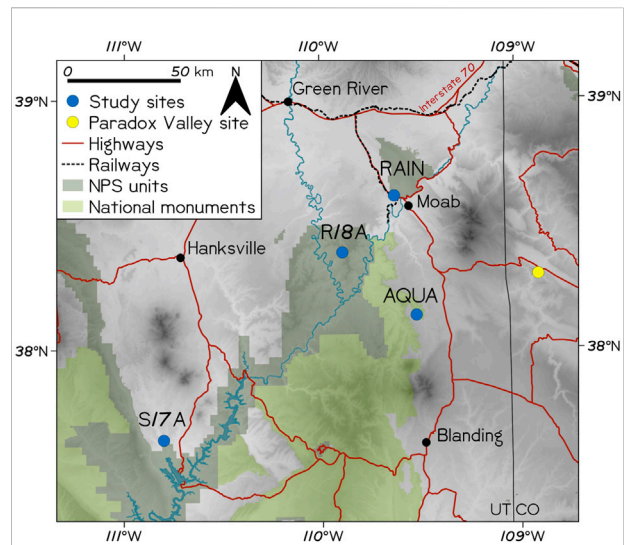


FIGURE 3
Map of field sites. RAIN: Rainbow Arch and Reference; AQUA: Aqueduct Arch; EarthScope TA stations S17A (Bullfrog Basin, UT) and R18A (Canyonlands National Park, UT); NPS: National Park Service.

resonance with peak arch velocity $\sim 40 \mu\text{m/s}$; events that can occur many times per hour (Figure 2). Railways, meanwhile, generate strong ground vibration in the frequency range of $\sim 5\text{--}50 \text{ Hz}$ (Quiros et al., 2016; Lavoué et al., 2020). While rail lines are less common in the Colorado Plateau than in other parts of the world, one located near Arches National Park, Union Pacific Railroad’s Cane Creek Branch, is actively used to transport potash from mining activity and uranium tailings from the bank of the Colorado River to a permanent storage area farther north (Denver and Rio Grande Western Railroad, 1970; United States Department of Energy, 2021). Helicopters, meanwhile, produce high-power airborne infrasound at frequencies that often align with the natural frequencies of arches, exciting resonance (Finnegan et al., 2021). These anthropogenic energy sources are thus able to excite resonance of rock arches and other landforms located nearby, with potentially important impacts on long-term rates of subcritical crack growth and structural degradation.

In this study, we collected continuous ambient vibration data from year-long measurements at two sites—one at the former location of a collapsed arch located near a busy highway and railroad, and another in an environment far from human activity—to compare and assess the effect of anthropogenic vibration on crack growth rates in arches. We used theory informed by field observations to compare hypothetical fracture growth in a conceptual rock arch placed in both environments and in scenarios with added anthropogenic vibration inputs. We then applied the results to understand the contribution of human activity to subcritical crack growth and progressive failure of rock landforms. We found that human-

induced vibration activity can have a notable effect on crack propagation in arches, especially for events with moderately large magnitude or high frequency of occurrence.

2 Field sites

We selected two sites for continuous ambient vibration recordings: the first located at the former site of Rainbow Arch, a ~4-m-long arch that collapsed during the winter of 2017–2018 (Figure 1), and the second located at Aqueduct Arch, spanning 24 m long (Figure 3; Geimer et al., 2022). Rainbow Arch, formed in Entrada Sandstone, was located near the Arches National Park Visitor Center parking lot near Moab, UT, ~175 m from US Highway 191 and ~285 m from an active railway. We measured continuous ground vibration on bedrock adjacent to the collapsed arch. Prior to collapse, we observed cracks running through the underside of the arch lintel, with a large fracture near the center where tensile stresses were anticipated to be greatest (Figure 1A; Moore et al., 2020). The other site, Aqueduct Arch, is formed in Wingate Sandstone and located far outside Moab, UT, largely isolated from human activity. At each site we deployed a 20-s Nanometrics Trillium Compact seismometer to measure continuous arch or ground vibrations sampled at 100 Hz. The seismometer recorded Aqueduct Arch vibration data from February 2017–May 2018 and from May 2019–June 2020 at the Rainbow Arch bedrock site. All seismometers were covered with a bucket to minimize wind buffeting. We did not observe a marked decrease in ground-borne vibration near the end of the Rainbow Arch site measurement due to COVID-19 pandemic lockdown responses (c.f. Lecocq et al., 2020). We also used 12 months of continuous seismic data from EarthScope Transportable Array (TA) stations S17A (Bullfrog Basin, UT) and R18A (Canyonlands National Park, UT) in 2008 for additional remote ambient vibration data, sites measured with buried Nanometrics Trillium 240 sensors sampling at 40 Hz. Arches National Park and the land therein have traditional and contemporary significance to many Native American tribes, including the Hopi, Navajo, Ute, Southern Paiute, and Zuni tribes (Stoffle et al., 2016). The landforms studied here represent a small fraction of thousands of documented arches in Utah (Van Bebbler, 2013), which are an iconic part of the landscape and attract millions of visitors each year (National Park Service, 2021).

3 Materials and methods

3.1 Annual arch peak ground velocity models

We began by creating peak ground velocity (PGV) magnitude-cumulative frequency (MCF) curves from continuous arch and reference vibration data. MCF curves have been traditionally used in seismology to relate the

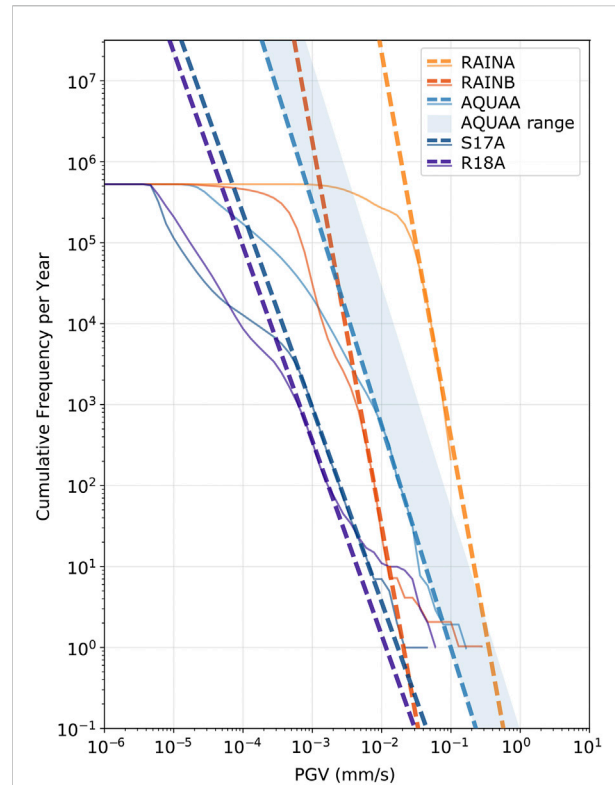


FIGURE 4

MCF curves of vibration data (solid lines) and models (dashed lines) for two arches and three bedrock reference sites. PGV is calculated at 1-min intervals from continuous ambient vibration data band-passed between 3 and 20 Hz. Aqueduct Arch (AQUAA) is the 'quiet' arch, located in a remote area between Moab and Blanding, UT, while Rainbow Arch (RAINA), located in Arches National Park, UT near a highway and railroad is the 'noisy' arch. Due to sub-optimal seismometer placement on Aqueduct Arch, the shaded area represents the range of PGV the arch experiences at the location of maximum modal displacement determined from numerical modeling (see Geimer et al., 2020; Finnegan et al., 2022). Reference data are from bedrock at the site of Rainbow Arch (RAINB) and from two TA stations located in remote areas of Southern Utah (S17A and R18A). The models are extended to estimate vibration amplitudes at longer recurrence intervals.

frequency of earthquakes of different magnitudes, where the relationship is a power law known as the Gutenberg-Richter Law (Gutenberg and Richter, 1954). Geologists in turn have adopted these curves for use in quantitative risk analysis for rockfall and other mass wasting events (e.g., Hungr et al., 1999). We used data from Aqueduct Arch and the former site of Rainbow Arch, along with TA stations to construct PGV MCF curves. As Rainbow Arch collapsed prior to the year-long data collection, we used arch-to-reference spectral ratios from limited ambient vibration recordings made on Rainbow Arch and nearby ground prior to its collapse in order to scale the reference MCF curve to a theoretical model representing arch vibrations over a year.

TABLE 1 Parameters used for crack growth calculations. Geometric values of length, thickness, and width developed empirically from arches in our study area.

Parameter	Symbol	Value
Length	L	5 m
Thickness	$h = 0.0012L^2 + 0.0512L + 0.0213$	0.3073 m
Width	$b = 2h$	0.6146 m
Gravitational acceleration	g	9.81 m/s ²
^a Density	ρ	2000 kg/m ³
^a Young's modulus	E	5 GPa
Moment of inertia	$I = \frac{bh^3}{12}$	0.001486 m ⁴
Non-dimensional frequency parameter	k	4.730
Gravitational self-weight	$W = bhg\rho$	3706 N/m
Fundamental frequency	$f_0 = \frac{k^2}{2\pi} \sqrt{\frac{EI}{bh\rho L^3}}$	20 Hz
Static stress	$\sigma_s = \frac{WL^2}{24Z}$, $Z = \frac{bh^2}{6}$	399 kPa
Horizontal stress at fundamental frequency	$\sigma = 2.477V\sqrt{E\rho}$	Varies [Pa] with velocity (m/s)
^b Characteristic grain size	d_g	1E-3 m
^c Fracture toughness	K_c	0.3 MPa m ^{1/2}
^d Paris' law exponent	m	60
Paris' law coefficient	$C \sim d_g K_c^{-m}$	2.359E+28 m (MPa m ^{1/2}) ^m
Cyclic stress amplitude	$\Delta\sigma = \frac{\sqrt{2}}{2} (\sigma_s + \sigma)$	Varies [Pa]
Stress intensity amplitude	$\Delta K_I = \Delta\sigma\sqrt{\pi a}$	Varies [MPa m ^{1/2}]
Critical crack length	$a_c = \frac{1}{\pi} \left(\frac{K_{Ic}}{\sigma_s}\right)^2$	0.17991 m
Initial crack lengths	$a_o \sim 0.920a_c$, $a_o \sim 0.975a_c$	0.16546 m, 0.17556 m

^aFrom Geimer et al. (2020).

^bFrom O'Sullivan (2003).

^cFrom Nara et al. (2012).

^dFrom Nara et al. (2017).

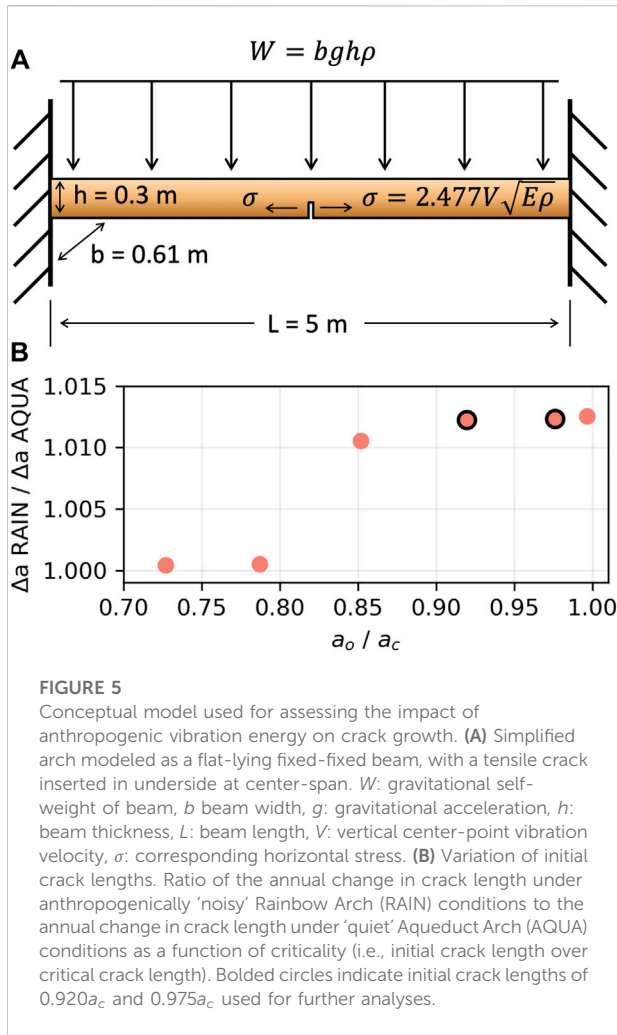
We processed vibration data in 1-h windows by detrending, demeaning, removing the instrument response, and filtering the data between 3 and 20 Hz for the ground reference datasets and 3–25 Hz for the arch datasets, in order to capture vibration at the natural frequencies (Geimer et al., 2020). We recorded the maximum instantaneous velocity within 1-min intervals over each hour and then repeated the process for all other hours in the year. Using the resulting PGV datasets, we constructed MCF curves for each station. Because the curves roll off for low-magnitude events, we fit the linear portion of each curve using a power law relationship to create a model of experienced PGV over the course of a year (Figure 4). We then applied these field results, in combination with simplified arch models that approximately describe arch geometry and modal behavior, to explore crack growth rates under different vibration conditions.

3.2 Conceptual crack growth model

We analyzed flat-lying arches (Figure 1) and used analytical solutions for a rectangular beam with two fixed ends (i.e., fixed-

fixed supported) to compute relevant static and dynamic structural conditions. To simplify our approach, we let the beam thickness, h , and width, b , scale empirically with span, L , using measurements from real arches in our study area to constrain these approximations (Table 1; Figure 5A). We additionally selected uniform and consistent material properties, based on calibration of dynamic models (Geimer et al., 2020) and analytical expressions for the eigenfrequencies of beam-like arches; see Table 1 for all parameters used in calculations.

We employed a pseudo-static approach to estimate dynamic stresses exerted on these beams. The approach begins by calculating the peak static stress under gravitational self-weight, then adding a load that reproduces the measured peak vibration velocity at the beam's fundamental frequency. To calculate the static stress, we applied a distributed load, W , consisting of the weight of the beam per unit length as: $W = bhg\rho$ (where g is gravitational acceleration and ρ is density), then calculated the horizontal static tensional stress at the center underside surface of the beam as: $\sigma_s = \frac{WL^2}{24Z}$, where Z is the section modulus determined as: $Z = \frac{bh^2}{6}$ (Gere et al., 1997).



Next, we determined the pseudo-static stress, σ , needed to generate a measured center-point vibration velocity at the beam’s fundamental frequency, using analytical expressions to derive a formulation for horizontal stress as a function of vertical velocity. First, noting that the deflection, δ , of a fixed-fixed beam under a uniform distributed load is $\delta = \frac{WL^4}{384EI}$ (Budynas et al., 2011), where I is the moment of inertia: $\frac{bh^3}{12}$, we rearranged to determine the load that would generate a measured deflection, and then substituted W into the above formula for stress, giving $\sigma = \frac{8Eh\delta}{L^2}$. Next, we calculated the fundamental frequency of a uniform fixed-fixed supported beam as $f_0 = \frac{k^2}{2\pi} \sqrt{\frac{EI}{bhpL^3}}$, where $k = 4.730$ (Kubojima et al., 2006), and the vibration velocity, V , at f_0 as $V = \pi f_0 \delta$. Rearranging the latter for δ and substituting into the expression for stress, we derived an analytical expression relating horizontal stress to fundamental vibration vertical velocity as: $\sigma = \frac{8EhV}{\pi f_0 L^2}$, which reduces to: $\sigma = 2.477V \sqrt{E\rho}$ for velocity in m/s (Table 1). This relationship demonstrates that stress is a linear function of velocity, independent of geometry of the beam.

We adopted a simplified kinetic fracture mechanics approach to compute and compare crack growth under different loading conditions. We began by varying initial crack lengths, starting with an initial crack length $\sim 75\%$ of the critical crack length—the length at which failure occurs—(Gdoutos, 2005), then progressively increased the initial crack length to assess crack growth under different stages of criticality. We calculated the mode I stress intensity amplitude $\Delta K_I = \Delta\sigma\sqrt{\pi a}$ (Anderson, 2005), which describes the variation of the stress field around a crack tip during a load cycle, where a is the edge crack length. We calculate $\Delta\sigma$, the cyclic stress amplitude, as $\frac{\sqrt{2}}{2}(\sigma_s + \sigma)$, using the root mean square to approximate the cyclic amplitude. While the static stress is the same for all experiments, the added dynamic stress varies based on the environment (i.e., anthropogenically ‘noisy’ or ‘quiet’) per results of our MCF analysis and added events, such as a given number of helicopter flybys or induced earthquakes. Other factors such as moisture and thermal cycling can strongly contribute to crack growth; however, here we solely address the vibration contribution. We employed Paris’ law, $\frac{da}{dN} = C(\Delta K_I)^m$, which relates the propagation rate of a crack ($\frac{da}{dN}$) to the stress intensity amplitude at the crack tip, in order to compute and compare crack growth cycle-by-cycle over 1 year and under exposure to different background vibration levels. C is the Paris’ law coefficient (here we used $C=2.359E+28 \text{ m} (\text{MPa m}^{1/2})^m$; Table 1) and m is the Paris’ law exponent (we used $m=60$; Nara et al., 2017), equivalent to the subcritical crack index n (Eppes and Keanini, 2017). Both parameters are empirically determined and depend on material and environmental factors; however, we use the resulting crack growth rates in a ratio, eliminating the analysis of absolute annual crack growth rates and dependency upon exact parameters for our idealized model.

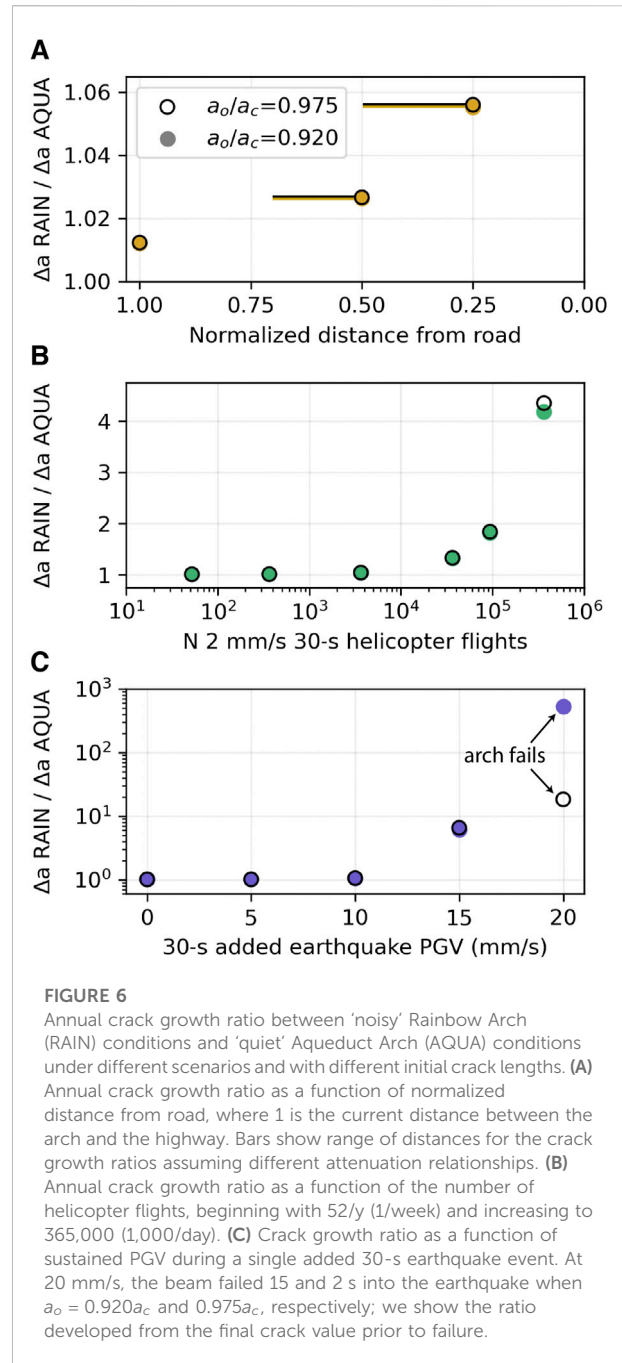
We used the MCF model values for Rainbow Arch and Aqueduct Arch to create a randomized sequence of PGV of the beam in each environment for every second over 1 year, sustaining the velocity and thus added dynamic stress for the number of cycles each second corresponding to the fundamental frequency of the beam (Table 1). In a first experiment, we compared annual crack growth in a 5-m arch (Table 1) between ‘noisy’ Rainbow Arch conditions and ‘quiet’ Aqueduct Arch conditions using these annual PGV series. We employed a range of initial crack lengths, analyzing crack growth at different stages of criticality. By integrating Paris’ law and solving for number of cycles to failure using an annual average additional stress from the ‘quiet’ environment, the initial crack lengths corresponded to the beam failing in ‘quiet’ conditions in roughly 200 ky, 20 ky, 2 ky, 200 y, 20 y, and 2 y. We then considered crack growth in hypothetical situations, such as if the distance between the highway and the arch decreased in the ‘noisy’ environment. For each successive halving of the distance between the arch and road, we doubled the PGV of the beam, following a general attenuation relationship of $V \propto d^{-1}$, where d

is the distance (Whiffin and Leonard, 1971). Our next scenario analyzed annual crack growth where we replaced a given number of 30-s vibration intervals with sustained 2 mm/s velocities, imitating potentially damaging landform vibration response under exposure to infrasound from helicopter flights (King, 2001; Finnegan et al., 2021). In the ‘noisy’ environment, we added regularly spaced helicopter flights, ranging from 52 times/y to 365,000 times/y. Arches and Canyonlands National Parks, containing thousands of arches, report several hundred helicopter overflights each year (National Park Service, 2020). Other parks, however, see far more flights, e.g., Rainbow Bridge National Monument and adjacent Glen Canyon National Recreation Area combine to permit >11,500 flights annually (National Park Service, 2020). We provide 365,000 flights/y as a hypothetical end member. Our final scenario considered annual crack growth in the model arch under exposure to a single added earthquake event. We replaced the last 30 s of the beam’s annual vibration velocity in the ‘noisy environment’ with sustained PGV between 5 and 20 mm/s for the added event, corresponding to ground motion amplification by the arches during earthquakes of different magnitudes and/or distances (Finnegan et al., 2022). We compared all resulting annual crack growth against growth in the anthropogenically ‘quiet’ environment, resulting in an annual crack growth ratio for an idealized arch in different environments.

4 Results

4.1 Annual arch velocity models

We produced annual PGV MCF curves for five sites: Rainbow Arch reference, reconstructed Rainbow Arch at center-span, TA stations S17A and R18A, and Aqueduct Arch west abutment (Figure 4). Aqueduct Arch ($F = 1.75 \times 10^{-3} V^{-2.75}$, where F is annual cumulative frequency and V is PGV in mm/s) and the TA station data represent an arch and reference sites, respectively, in an anthropogenically ‘quiet’ area, while Rainbow Arch ($F = 1.00 \times 10^{-8} V^{-4.75}$) and its reference data represent an arch and bedrock site, respectively, in a ‘noisy’ area. We then used the PGV MCF curves to produce randomized annual PGV series of model arch vibration. Due to lack of safe access, the seismometer placement at Aqueduct Arch did not record the peak vibration amplitude of the fundamental and many higher-order modes. To address this, we multiplied PGV values by a factor of 5, scaling measured data to the maximum displacement at the fundamental mode determined from numerical modeling (see Geimer et al., 2020; Finnegan et al., 2022). We used the resulting curve to represent the true PGV likely experienced at the location of maximum modal displacement of Aqueduct Arch (i.e., center-span). The Rainbow Arch dataset suffered from infrequent measurements, so we used the Rainbow Arch



reference curve, along with measured arch-to-reference amplification factor of ~20 to confirm the Rainbow Arch model accurately represented arch vibration over a year (Figure 4). The resulting curves indicate that Aqueduct Arch experiences vibration amplitudes in the range of 0.01–0.1 mm/s each year roughly 1,000 times less frequently than Rainbow Arch. We produced similar results comparing Rainbow Arch reference data near Highway 191 with the remote TA station reference data. These confirmed that there is a substantial difference in

ground, and therefore arch, vibration experienced between anthropogenically ‘noisy’ and ‘quiet’ areas.

4.2 Annual crack growth

Our first crack growth comparison analyzed fracture propagation over 1 year between beams in ‘noisy’ Rainbow Arch conditions, with exposure to significant road and rail traffic, and in ‘quiet’ Aqueduct Arch conditions, where there is little human activity. We varied initial crack lengths to explore crack growth at various stages of criticality (Figure 5B), and for subsequent comparisons, we selected an initial crack length (a_o) that was 97.5% the length of the critical crack (a_c), and a second initial crack length 92.0% of the critical crack length (Table 1). These initial crack lengths correspond to the beam failing in ‘quiet’ conditions in ~20 and ~200 y, respectively. In 1 year with $a_o = 0.975a_c$, the ratio of the crack growth between ‘noisy’ and ‘quiet’ conditions is 1.0124, while in 1 year with $a_o = 0.920a_c$, the annual crack growth ratio is 1.0122., thus ~1% longer for both initial crack lengths.

We next created a scenario where we analyzed the annual crack growth ratio under ‘noisy’ and ‘quiet’ conditions and considered if the highway and railroad were closer to the arch (Figure 6A). With each halving of the distance between the arch and road, we doubled the PGV of the beam. When the distance between the arch and roads was halved once, the crack growth in 1 year was ~2.5% longer than in ‘quiet’ conditions for both initial crack lengths. When the distance was halved again, the crack growth in 1 year was ~5.5% longer than in ‘quiet’ conditions for both initial crack lengths.

We then considered crack growth where in the ‘noisy’ conditions, we replaced regularly spaced 30-s vibration intervals with 2 mm/s sustained velocities to imitate arch vibration response to helicopter flights (Figure 6B). The crack began to grow substantially longer in the ‘noisy’ environment—~33% longer than in the ‘quiet’ conditions for both initial crack lengths—when 100 flights/day were added. When 94,000 total flights were added, the annual crack growth was ~83% longer than in ‘quiet’ conditions for both initial crack lengths. At 365,000 added helicopter flights, the annual crack growth ratio diverged slightly for the different initial conditions: crack growth in 1 year was ~4.2 and ~4.4 times the growth in ‘quiet’ conditions when $a_o = 0.920a_c$ and $0.975a_c$, respectively.

The final scenario we considered was replacing the last 30 s of the ‘noisy’ environment yearly PGV sequence with a single earthquake lasting 30 s (Figure 6C). When we added an earthquake which generated beam PGV of 10 mm/s, the crack growth in 1 year was ~5.5% longer than in ‘quiet’ conditions for both initial crack lengths. With an added earthquake where the beam experienced sustained PGV of 15 mm/s for 30 s, the crack grew ~6.1 and ~6.5 times the growth in ‘quiet’ conditions when $a_o = 0.920a_c$ and $0.975a_c$, respectively. For PGV of 20 mm/s, the

beam failed 15 s into the earthquake with the less critical initial conditions and 2 s into the earthquake with the more critical initial conditions.

5 Discussion

Our results demonstrate that moderate-magnitude single events (order cm/s) or smaller-magnitude events (order mm/s) that occur at high frequencies resulting from human activity have the ability to contribute to crack growth in, and thus structural degradation of, naturally occurring rock arches. We also found that the initial criticality of the crack, if in the range of ~0.90–0.99 (a_c) (Figure 5A), only produced different annual crack growth ratios when events became larger or more frequent. This indicates that the crack growth metrics we computed in 1 year under different anthropogenic scenarios are valid for a range of initial crack lengths, which is helpful as *in-situ* crack lengths in arches are not always simple nor safe to measure.

For the cycle-by-cycle calculations, we employed the simplest mode I stress intensity amplitude, $\Delta K_I = \Delta\sigma\sqrt{\pi a}$. We confirmed that this analytical solution appropriately estimated the stress intensity amplitude at the crack tip through comparison to results from numerical estimates of K_I . We used a 3D photogrammetric model of Rainbow Arch (from Geimer et al., 2020), defined material properties and boundary conditions, and inserted a crack in the underside of the arch at the location where the tensile crack existed *in-situ* into the fracture analysis computational program FRANC3D (fracanalysis.com). We then added a distributed load corresponding to the gravitational self-weight plus the pseudo-static stress from additional vibration velocities and calculated the stress intensity factor. K_I calculated with the simple analytical solution was within 10–30% of the results from FRANC3D. For smaller velocities, and thus smaller additional stresses, the analytical solutions slightly overpredicted K_I compared to numerical results, but for velocities >0.5 mm/s, the analytical solution underpredicted K_I , indicating that the simplest stress intensity factor possibly underpredicts crack growth rates at larger velocities. We additionally tested more complex analytical solutions for K_I , such as the single-edge-cracked three-point bend specimen (Gdoutos, 2005); however, K_I values using this solution were orders of magnitude larger than both the FRANC3D and simplest analytical solution, so we did not use these values for the annual crack growth calculations.

Comparing crack growth in a conceptual beam model over the course of a year between Rainbow Arch and Aqueduct Arch environments, we found the crack grew ~1% more in the ‘noisy’ conditions than in ‘quiet’ conditions for both initial crack lengths analyzed. This indicates that while road and railway vibrations may have aided in the structural degradation and collapse of

Rainbow Arch, the contributions were likely comparably small. When halving the distance between the arch and the road, the annual crack growth ratio doubled. However, we note that doubling the PGV with each halving of the distance between the arch and road may underestimate the change in vibration velocity. Depending on the road setting, surface, and condition, along with the speed and types of vehicles, the attenuation relationship between PGV and distance can vary from $V \propto d^{-1}$. King (2001) and King and DeMarco (2003) report road vibration attenuation correlations of $V \propto d^{-1.6}$ to $V \propto d^{-2.3}$. Additionally, different attenuation relationships are possible for different path materials such as soft soil or bedrock. While we used $V \propto d^{-1}$ as a lower bound, we show a possible range in Figure 6A for the crack growth ratio given attenuation at $V \propto d^{-2}$. Trains caused the largest vibration of Rainbow Arch, and although they passed only ~10 times per week, it is possible that an increase in train traffic could result in a larger effect on crack growth at a nearby arch.

In considering the contribution of helicopter-sourced infrasound to crack growth, results of our conceptual models indicate that compared to ‘quiet’ environments, no substantial additional crack growth occurs until there are >~100 flights/day (~37,000 flights/y) at sustained 2 mm/s PGV (Figure 6B). For rock arches in Arches and Canyonlands National Parks, which are exposed to several hundred helicopter overflights each year (National Park Service, 2020), our results suggest this volume of flights is not likely to modify nominal crack growth rates. However, arches in other areas, such as Rainbow Bridge, can experience >11,500 flights annually (National Park Service, 2020). Additionally, Grand Canyon National Park permits ~94,000 flights/y (National Park Service, 2011), and the park contains many prominent landforms such as rock buttes, fins, and towers, in addition to arches. Our results demonstrate that increased crack propagation at these very high rates of occurrence could cause unwanted degradation of culturally valued rock landforms in these areas. We note that PGV levels of 2 mm/s are greater than those measured experimentally on rock towers and arches by Finnegan et al. (2021) but less than measured by King (2001) on rock towers, and we suggest these represent a relatively high level of excitation from close-proximity helicopter flights; lower PGV values are anticipated for distant overflights (Finnegan et al., 2021).

Considering the contribution of seismic activity, our results show that moderate-magnitude earthquakes can cause considerable crack growth compared to ‘quiet’ environments (Figure 6C). Adding a 30-s event with sustained beam PGV of 15 mm/s, we found the crack grew >6 times the annual crack growth under ‘quiet’ conditions. At sustained 20 mm/s PGV for 30 s, the beam failed, indicating possible arch collapse given similar initial conditions and exposure to these PGV levels. Although southern Utah, where many sandstone arches are located, is not a highly seismically active area, a nearby brine injection facility installed in the 1990s in Paradox Valley, CO

(Figure 1) has induced multiple $M4+$ earthquakes in recent years (Block et al., 2015), with one $M_{w}4.5$. Given the natural amplification of ground motion by arches, at a distance of 85 km from the epicenter, many of these landforms could have experienced PGV of 10 mm/s during the $M_{w}4.5$ earthquake (Finnegan et al., 2022). Our results suggest that annual crack growth, in an arch similar to our conceptual model experiencing this earthquake, was likely ~5.5% greater than in ‘quiet’ conditions without this earthquake.

Our final two scenarios where we added helicopter flights and an earthquake can also be generally applied to other situations, such as roadwork and vibrations created by construction equipment, as well as different types of blasting or possibly even sonic booms. Additionally, in considering the effects of added anthropogenic energy on arches of different sizes, we expect that smaller arches may be more susceptible to increased crack growth given an added vibrational energy than larger arches. We base this expectation on the comparably larger addition of pseudo-static stress for small arches where gravitational self-weights are lower than in larger arches with greater self-weights. As the added pseudo-static stress is independent of geometry it is unchanged from arch to arch, whereas the gravitational self-weight of an arch increases with growing dimensions, and thus larger arches may be less affected by added anthropogenic energy than smaller arches.

Our model provides valuable insight into the effects of anthropogenic activity on structural degradation of rock arches; however, some limitations are present. As the Paris’ law coefficient and exponent we selected were determined empirically and depend on factors such as temperature (Nasseri et al., 2009) and humidity (Nara et al., 2012), there can be significant variation in the absolute annual crack growth given a range of possible values. We limited our study to calculating annual crack growth ratios as a relative metric, in order to eliminate the need for precise parameters that would require laboratory testing, or an extensive sensitivity test of our model. Additionally, our model employs a flat-lying fixed-fixed beam to study crack growth in an idealized arch. Arches shaped in the form of an inverted catenary, for instance, experience a different stress state (Moore et al., 2020) and thus, our model may not be applicable in all cases and geometries. Our model also solely examines crack growth of a single crack inserted in the underside of a beam at center-span: pre-existing bedding structures and crack networks in the rock mass can control how failure occurs and add a greater level of complexity to non-theoretical crack growth (Wong et al., 2001).

More detailed studies investigating the cumulative effects of all stressors—both anthropogenic and natural—on crack growth, along with incorporating elements such as precise geometry and existing material flaws, are necessary to comprehensively understand crack growth in arches. Previous site-specific studies such as those analyzing seismic signatures of arch collapse (Galea et al., 2018), stability and stress assessments of arches (Moore et al., 2020; Leucci

et al., 2021), and studies that explore arch erosion (Říhošek et al., 2018) will be helpful in this regard. Notwithstanding limitations of our model, many arches do contain observable cracks and are failing under subcritical stress conditions. Rainbow Arch is one example where a crack reached its critical length, resulting in complete collapse of the arch. Our model and results may aid conservation management decisions regarding the current and long-term impacts of various human activities on the structural health of rock arches, as well as other rock landforms.

6 Conclusion

Using a theoretical model informed by natural arch and ground vibration data, we compared annual crack growth in an idealized beam-like arch experiencing anthropogenic vibrations to annual crack growth under conditions with little-to-no added anthropogenic energy. We found that added anthropogenic vibration energy can have a minor but distinct effect on annual crack growth in arches, resulting in a crack that is approximately 1% longer than in natural, ‘quiet’ conditions. We saw an especially larger impact on the annual crack growth ratio given a moderate-magnitude shaking event (~15 mm/s) or higher-frequency (>37,000) of lower-magnitude (2 mm/s) events. We conclude that given this effect, human activity can have a significant contribution on crack growth over short and long periods of time, potentially affecting the lifespan of these culturally valued landforms. Our study informs questions on the impacts human activity has on the rate of degradation of natural landforms, and provides a quantitative approach to assess a range of scenarios designed to be useful in land conservation management practice.

Data availability statement

The datasets generated for Aqueduct Arch and Rainbow Arch and Reference for this study are available at the International Federation of Digital Seismograph Networks (FDSN): https://doi.org/10.7914/SN/5P_2013. The Transportable Array data analyzed for this study are available from FDSN: <https://doi.org/10.7914/SN/TA>.

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Author contributions

RF and JM conceived of the project. RF, PG, and JM collected field data. RF wrote the manuscript, with contributions from JM and PG.

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Conflict of interest

The authors declare that the research was conducted in the absence of any commercial or financial relationships that could be construed as a potential conflict of interest.

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