



Critical Response of Nonlinear Base-Isolated Building Considering Soil-Structure Interaction Under Double Impulse as Substitute for Near-Fault Ground Motion

Hiroki Akehashi¹, Kotaro Kojima², Kohei Fujita¹ and Izuru Takewaki^{1*}

¹ Department of Architecture and Architectural Engineering, Graduate School of Engineering, Kyoto University, Kyotodaigaku-Katsura, Kyoto, Japan, ² Faculty of Design and Architecture, Kyoto Institute of Technology, Kyoto, Japan

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*Correspondence:

Izuru Takewaki
takewaki@archi.kyoto-u.ac.jp

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The critical nonlinear response considering soil-structure interaction is investigated for a base-isolated building under a double impulse as a substitute for near-fault earthquake ground motions. The complicated model of the nonlinear base-isolated building considering soil-structure interaction is first modeled as a two-degree-of-freedom (2DOF) system (SDOF superstructure and base-isolation story) on a swaying-rocking spring-dashpot system. Then the 2DOF system on a swaying-rocking spring-dashpot system is transformed into an SDOF system on a swaying-rocking spring-dashpot system by neglecting the mass on the base-isolation story. Finally the SDOF system on a swaying-rocking spring-dashpot system is further transformed into an SDOF system by neglecting the mass and moment of inertia of the base mat. Since an explicit expression had been derived in the previous paper on the maximum elastic-plastic response of an SDOF damped bilinear hysteretic structure subjected to the “critical double impulse input” causing the maximum response for variable interval of impulses with the input level kept constant, this expression is applied to the finally transformed SDOF system. The transformation of structural viscous damping in different elements is another new aspect in this paper. The reliability and accuracy of the proposed simplification methodology are investigated by comparing with the results by the time-history response analysis under the critical double impulse and the one-cycle sine wave as a representative of the main part of the near-fault earthquake ground motion.

Keywords: critical excitation, soil-structure interaction, base-isolated building, elastic-plastic response, bilinear hysteresis, near-fault ground motion, resonance

INTRODUCTION

A great deal of useful records from recent earthquakes enabled a clear classification of earthquake ground motions in view of their characteristics (Abrahamson et al., 1998). Especially this observation is being accelerated due to dense arrangement of high-performance measurement systems. One is a short-duration intensive ground motion characterized by a near-fault ground motion and another one is a long-period and long-duration ground motion mostly with a far fault (see Takewaki et al., 2011). It is widely understood in the field of earthquake resistant

design of structures that surface-soil properties influence greatly earthquake ground motions at ground surface. For this reason, the surface soil types (soil, rock) are important factors for such classification as well as fault mechanisms. On the effects of near-fault ground motions on structural inelastic responses, various viewpoints have been provided (for example Bertero et al., 1978; Hall et al., 1995; Sasani and Bertero, 2000; Mavroeidis and Papageorgiou, 2003; Alavi and Krawinkler, 2004; Makris and Black, 2004; Mavroeidis et al., 2004; Kalkan and Kunnath, 2006; Khaloo et al., 2015). Fling-step (parallel to the fault plane) and forward-directivity (normal to the fault plane) are often discussed and used recently for designating such near-fault ground motions. Representatives of recent related earthquake ground motions are Northridge earthquake in 1994, Hyogoken-Nanbu (Kobe) earthquake in 1995, Chi-Chi (Taiwan) earthquake in 1999 and Kumamoto earthquake in 2016. Recently strong earthquakes occurred in Italy, e.g., the 2009 L'Aquila (Di Sarno et al., 2011) and 2016–2017 Central Italy earthquakes. Such seismic events had significant near field effects.

For simple but essential discussion, two or three half-cycle sine waves have been extracted from fling-step and forward-directivity motions (see **Figure 1**). As the first step, many engineers discussed mainly the elastic response under the near-fault ground motions. Since there are many parameters (e.g., duration and amplitude of pulse, predominant period, ratio of pulse frequency to structure natural frequency) and the corresponding numerical parametric analysis is extremely complicated for elastic-plastic response, the treatment of only elastic response was reasonable. However, the investigation on elastic-plastic response is inevitable from the reliable damage assessment and enhancement of true safety of structures.

Kojima and Takewaki (2015a) introduced an innovative approach using the double impulse as expressed in **Figure 2A** to overcome such complex difficulty. The double impulse substitutes for the main effect of the fling-step near-fault ground motion and the explicit maximum elastic-plastic response was obtained in a structure under the “critical double impulse” using the smart energy balance law. The “critical input” is closely related to the critical excitation method (see Drenick, 1970; Takewaki, 2007). Since only the free vibration appears under such double impulse, their approach enabled the simple expression of complex elastic-plastic response. The introduction of the triple impulse enabled the successful application of the methodology for the fling-step motion to a more realistic forward-directivity motion by Kojima and Takewaki (2015b) (see **Figure 2B**). This approach using impulses was further extended to long-period, long-duration ground motions in terms of multi impulses (Kojima and Takewaki, 2015c).

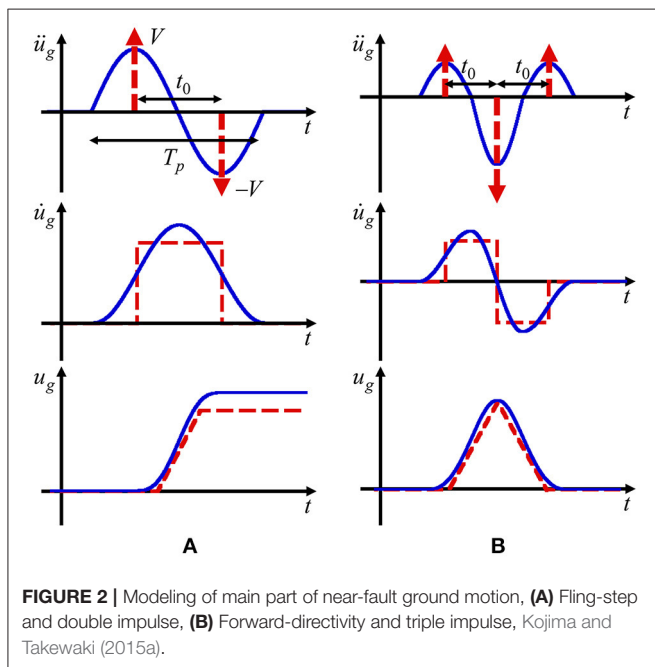
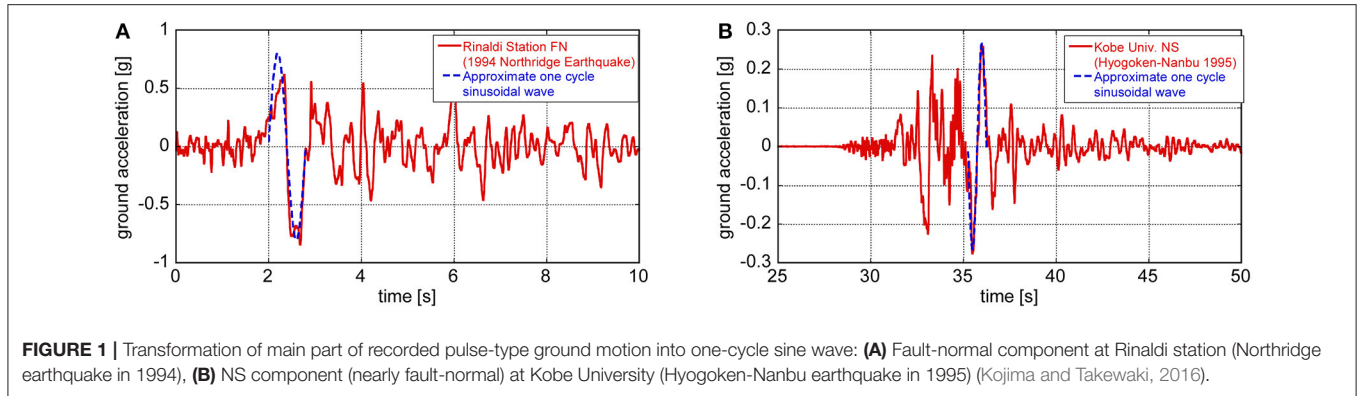
In the last century, the field of earthquake-resistant design was developed successfully and the resonance played a key role in the damage analysis of structures (e.g., Elnashai and Di Sarno, 2008). Once an input level is specified, the resonant equivalent frequency is required to be analyzed by changing the input frequency of a sine wave parametrically (Caughey, 1960a,b; Iwan,

1961, 1965a,b; Roberts and Spanos, 1990; Liu, 2000). Following this approach, Luco (2014) presented an interesting result on nonlinear steady-state response for a base-isolated building with nonlinear isolators on flexible ground. He employed the equivalent linearization technique to draw the resonance curve which requires the repetition in the determination of equivalent parameters and the sweeping of excitation frequency. On the other hand, such computation without repetition is realized by the double impulse. The double impulse enables the smart capture of the resonance through an energy balance law and the timing of the second impulse can be obtained as the time at which the zero restoring force is attained after the first impulse. The maximum elastic-plastic response after the first impulse can be obtained by transforming the initial kinetic energy given by the first impulse into the sum of elastic strain energy, hysteretic energy and the damping energy (Kojima et al., 2017).

In this paper, the critical nonlinear response is investigated for a base-isolated building on flexible ground under a double impulse as a good substitute for near-fault earthquake ground motions. The complex model of the nonlinear base-isolated building considering soil-structure interaction is first modeled using a two-degree-of-freedom (2DOF) system (SDOF superstructure and base-isolation story) on a swaying-rocking spring-dashpot system which represents the flexibility and damping of ground. Then the 2DOF system on a swaying-rocking spring-dashpot system is transformed into an SDOF system on a swaying-rocking spring-dashpot system by neglecting the mass on the base-isolation story. Finally the SDOF system on a swaying-rocking spring-dashpot system is further transformed into an SDOF system by neglecting the mass and moment of inertia of the base mat. Since an explicit expression had been derived in the previous work (Akehashi et al., 2018) on the maximum elastic-plastic response of an SDOF damped structure with bilinear hysteresis under the “critical double impulse input” which causes the maximum response for variable impulse interval with the input level kept constant, this expression is applied to the finally transformed SDOF system. The transformation of structural viscous damping in different elements is another new aspect in this paper. The reliability and accuracy of the proposed simple methodology are investigated by comparing with the results by the time-history response analysis to the critical double impulse and the one-cycle sine wave as a representative of the main part of the near-fault earthquake ground motion.

BASE-ISOLATED BUILDING CONSIDERING SOIL-STRUCTURE INTERACTION AND ITS MODELING INTO 2DOF MODEL ON SWAYING-ROCKING SPRING-DASHPOT SYSTEM

Consider a base-isolated building on ground as shown in **Figure 3**. The superstructure is modeled as an SDOF system and the stiffness and damping of the ground are represented by a swaying-rocking spring-dashpot system. The base-isolation story



TRANSFORMATION OF 2DOF SYSTEM ON SWAYING-ROCKING SPRING-DASHPOT SYSTEM INTO SDOF SYSTEM ON SWAYING-ROCKING SPRING-DASHPOT SYSTEM

The 2DOF system consisting of an SDOF superstructure and a base-isolation story on rigid ground is transformed into an SDOF system as shown in **Figure 5** by neglecting the degree of freedom just above the base-isolation story. Then the total system is the SDOF system on a swaying-rocking spring-dashpot system. In this section, the 2DOF system on rigid ground is transformed into an SDOF system. Let u_e , k_e , c_e denote the displacement of the transformed SDOF mass (=superstructure mass), the stiffness and damping coefficient of the transformed SDOF model.

The procedure of neglecting the mass just above the base-isolation story and neglecting the corresponding degree of freedom is explained.

The dynamic equilibrium at the degree of freedom just above the base-isolation story can be expressed by

$$k_U u_U + c_U \dot{u}_U = k_I u_I + c_I \dot{u}_I = k_e u_e + c_e \dot{u}_e \quad (1)$$

The stiffness k_e of the equivalent SDOF model can be derived based on the series spring model.

$$\frac{1}{k_e} = \frac{1}{k_U} + \frac{1}{k_I}, \quad u_e = u_U + u_I \quad (2)$$

On the other hand, the damping coefficient c_e of the equivalent SDOF model can be related to the respective damping coefficients c_U , c_I . The following equation can be obtained from the Fourier transform of Equation (1).

$$\frac{1}{k_U + i\omega c_U} + \frac{1}{k_I + i\omega c_I} = \frac{1}{k_e + i\omega c_e} \quad (3)$$

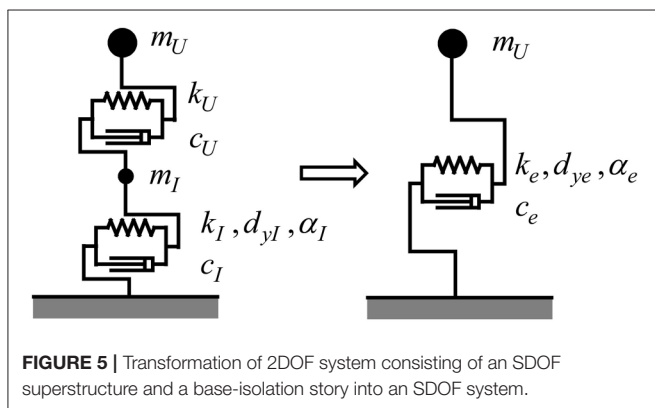
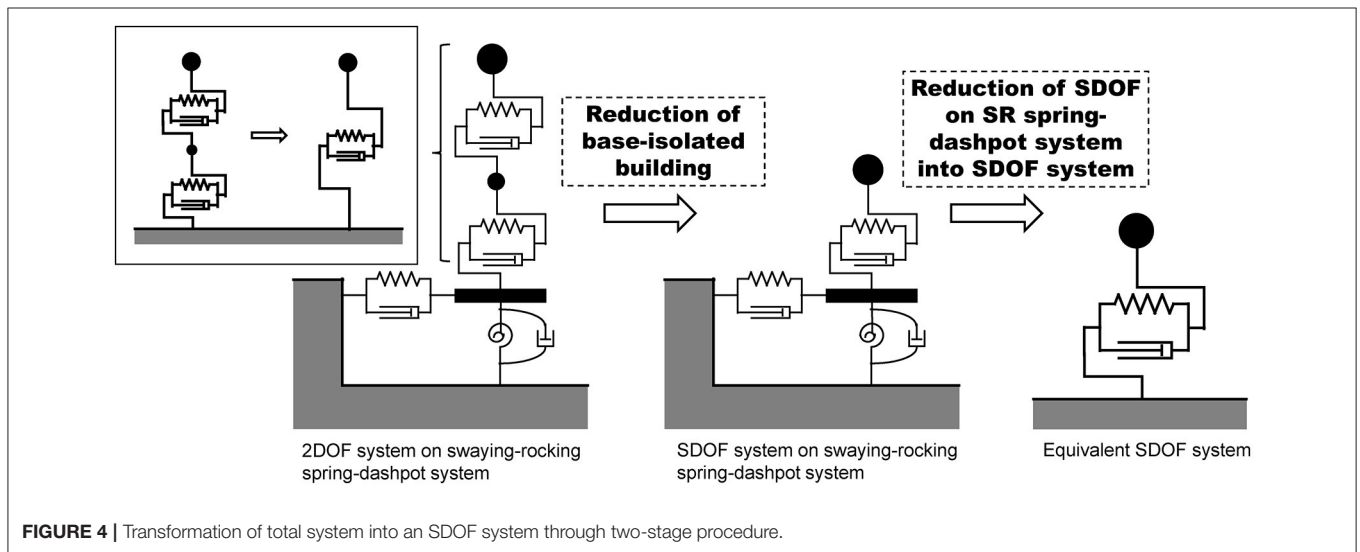
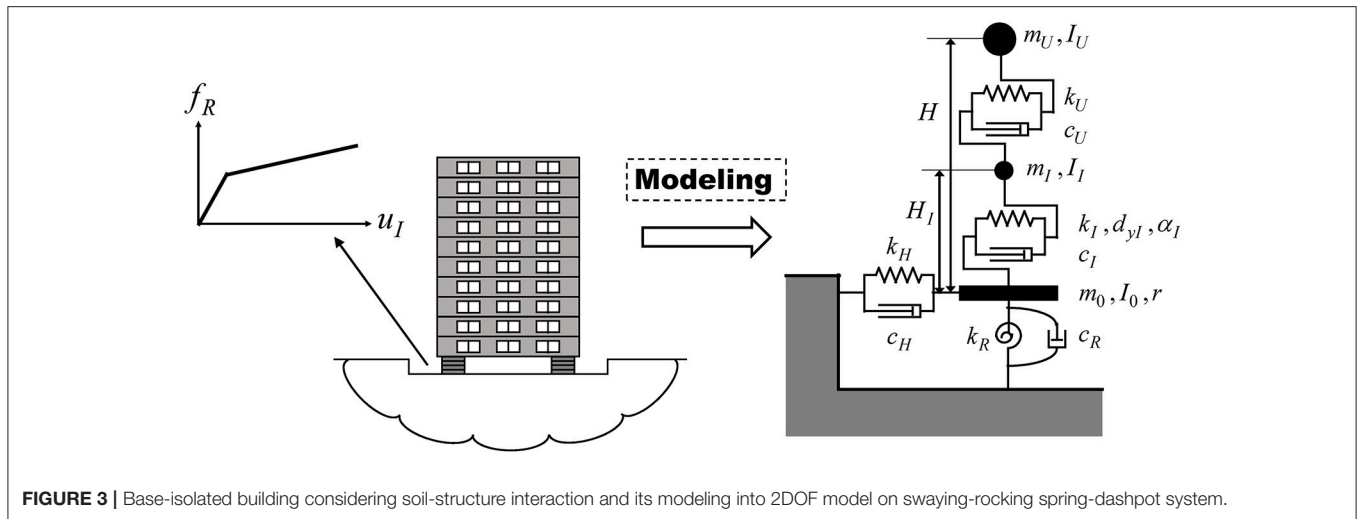
From Equation (3), c_e can be obtained as follows.

$$c_e = \frac{(k_U c_I + k_I c_U)(k_U + k_I) - (k_U k_I - \omega_e^2 c_U c_I)(c_U + c_I)}{(k_U + k_I)^2 + \omega_e^2 (c_U + c_I)^2} \quad (4)$$

where $\omega_e = \sqrt{k_e/m_U}$.

is assumed to consist of lead rubber bearings and is modeled by a shear spring with bilinear hysteresis. Then the total system is a 2DOF shear building model supported by the swaying-rocking spring-dashpot system. This total system is finally transformed into an SDOF system as shown in **Figure 4**. The masses of the superstructure, the base-isolation story and the base mat are denoted by m_U , m_I , m_0 and the corresponding mass moments of inertia are denoted by I_U , I_I , I_0 . Let d_{yI} and α_I denote the yield deformation and the post-yield stiffness ratio to the initial stiffness at the base-isolation story.

Let u_U , u_I , u_H , θ_R denote the deformation of the superstructure, the deformation of the base-isolation story, the swaying displacement and the angle of rotation of the base mat. The stiffnesses and damping coefficients of the superstructure and the base-isolation story are denoted by k_U , k_I , c_U , c_I . The swaying-rocking stiffnesses and damping coefficients of the spring-dashpot system of the ground are denoted by k_H , k_R , c_H , c_R . H is the height of the equivalent mass from the base mat.



From the static equilibrium at the yielding point, the yield deformation is expressed by

$$d_{ye} = d_{yI} + \frac{k_I d_{yI}}{k_U} = \frac{k_I}{k_e} d_{yI} \tag{5}$$

From $1/(\alpha_e k_e) = 1/(\alpha_I k_I) + 1/k_U$, the second stiffness ratio of the equivalent SDOF model can be expressed by

$$\alpha_e = \alpha_I \{1 + (k_I/k_U)\} / \{1 + (\alpha_I k_I)/k_U\} \tag{6}$$

TRANSFORMATION OF SDOF SYSTEM ON SWAYING-ROCKING SPRING-DASHPOT SYSTEM INTO SDOF SYSTEM

By neglecting the degrees of freedom at the base mat (the mass and moment of inertia of the base mat are neglected), the SDOF system on a swaying-rocking spring-dashpot system can be transformed into an equivalent SDOF system as shown in Figure 6.

Let u and u_s denote the total displacement of the equivalent SDOF system and the deformation of the equivalent SDOF system (displacement without swaying-rocking component). k and c are the stiffness and damping coefficient of the equivalent SDOF system.

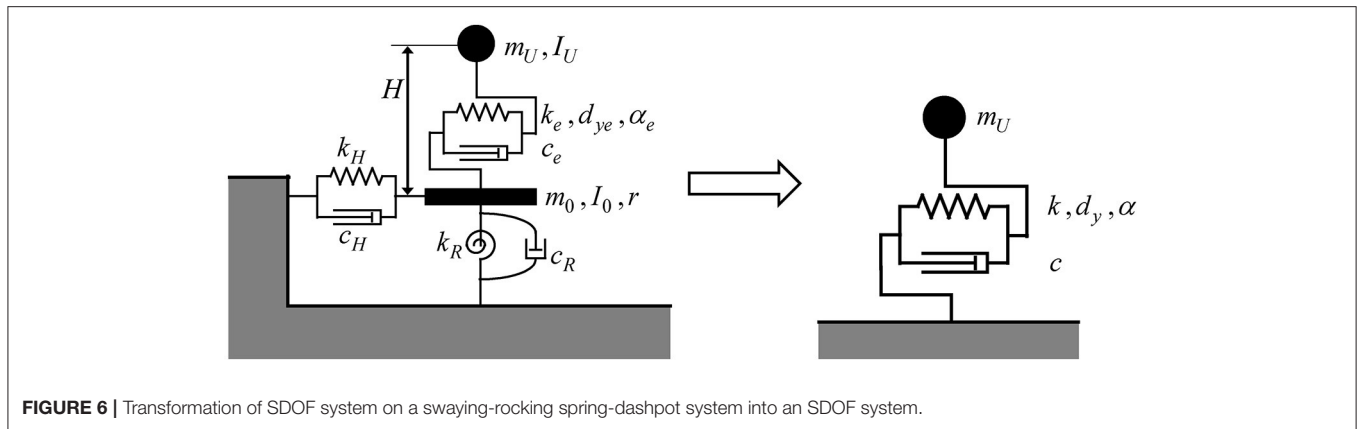


FIGURE 6 | Transformation of SDOF system on a swaying-rocking spring-dashpot system into an SDOF system.

The equation of motion of the SDOF system on a swaying-rocking spring-dashpot system can be expressed by

$$m_U \ddot{u} + c_e \dot{u}_s + k_e u_s = 0 \tag{7}$$

The horizontal and rotational dynamic equilibriums at the base mat can be described by

$$k_e u_s + c_e \dot{u}_s - (k_H u_H + c_H \dot{u}_H) = 0 \tag{8}$$

$$k_e u_s + c_e \dot{u}_s - (k_R \theta_R + c_R \dot{\theta}_R)/H = 0, \tag{9}$$

where u is the following total displacement of the equivalent SDOF system.

$$u = u_s + u_H + \theta_R H \tag{10}$$

The equation of motion of the equivalent SDOF system can be expressed by

$$m_U \ddot{u} + c \dot{u} + k u = 0 \tag{11}$$

Equations (8, 9) are rearranged into

$$k_e u_s + c_e \dot{u}_s = k_H u_H + c_H \dot{u}_H = (k_R \theta_R + c_R \dot{\theta}_R)/H = k u + c \dot{u} \tag{12}$$

The static series-spring assumption leads to

$$\frac{1}{k} = \frac{1}{k_e} + \frac{1}{k_H} + \frac{H^2}{k_R} \tag{13}$$

On the other hand, the dynamic series-spring assumption provides

$$\frac{1}{k + i\omega c} = \frac{1}{k_e + i\omega c_e} + \frac{1}{k_H + i\omega c_H} + \frac{H^2}{k_R + i\omega c_R} \tag{14}$$

The damping coefficient of the equivalent SDOF system can be derived as

$$c = \frac{(k_e c_g + k_g c_e)(k_e + k_g) - (k_e k_g - \omega_1^2 c_e c_g)(c_e + c_g)}{(k_e + k_g)^2 + \omega_1^2 (c_e + c_g)^2}, \tag{15}$$

where $\omega_1 = \sqrt{k/m_U}$,

$$k_g = \frac{(k_H \frac{k_R}{H^2} - \omega_1^2 c_H \frac{c_R}{H^2}) (k_H + \frac{k_R}{H^2}) + \omega_1^2 (k_H \frac{c_R}{H^2} + \frac{k_R}{H^2} c_H) (c_H + \frac{c_R}{H^2})}{(k_H + \frac{k_R}{H^2})^2 + \omega_1^2 (c_H + \frac{c_R}{H^2})^2} \tag{16}$$

$$c_g = \frac{(k_H \frac{c_R}{H^2} + \frac{k_R}{H^2} c_H) (k_H + \frac{k_R}{H^2}) - (k_H \frac{k_R}{H^2} - \omega_1^2 c_H \frac{c_R}{H^2}) (c_H + \frac{c_R}{H^2})}{(k_H + \frac{k_R}{H^2})^2 + \omega_1^2 (c_H + \frac{c_R}{H^2})^2} \tag{17}$$

From the static equilibrium at the yielding point, the yield deformation can be expressed by

$$d_y = d_{ye} + k_e d_{ye} (1/k_H + H^2/k_R) = \frac{k_e}{k} d_{ye} \tag{18}$$

From $1/(\alpha k) = 1/(\alpha_e k_e) + 1/k_H + H^2/k_R$, the second stiffness ratio after yielding of the equivalent SDOF system can be expressed by

$$\alpha = \alpha_e \frac{1 + \frac{k_e}{k_H} + \frac{k_e H^2}{k_R}}{1 + \frac{\alpha_e k_e}{k_H} + \frac{\alpha_e k_e H^2}{k_R}} \tag{19}$$

EXPLICIT EXPRESSION ON CRITICAL ELASTIC-PLASTIC RESPONSE UNDER DOUBLE IMPULSE

In the previous work (Akehashi et al., 2018), an explicit expression on the peak deformation of a damped SDOF system of bilinear hysteresis under the critical double impulse has been derived. That expression is used in this paper for predicting the response of a base-isolated building on ground. In particular the maximum elastic-plastic deformation of the base-isolation story under the critical double impulse is obtained by relating that deformation with the total deformation of the equivalent SDOF model.

A damped SDOF system of bilinear hysteresis is considered which has the mass m_U , initial stiffness k and damping

coefficient c . This system is subjected to the double impulse $\ddot{u}_g(t) = V\delta(t) - V\delta(t - t_0)$ of ground acceleration as shown in **Figure 7**. This SDOF system is the same as that in **Figure 6**. V is the given velocity value of the first and second impulses. In addition, t_0 is the time interval between two impulses and this value is treated as a variable in the problem of finding the critical double impulse. The ratio of the post-yield stiffness to the initial one is designated by α ($\alpha > 0$). The yield deformation and force are expressed by d_y and f_y . The bilinear hysteretic restoring-force characteristic is described by $f(u)$. Let $\omega_1 = \sqrt{k/m_U}$, $T_1 = 2\pi/\omega_1$, $\omega'_1 = \sqrt{1-h^2}\omega_1$, $T'_1 = 2\pi/\omega'_1$ denote the undamped natural circular frequency, the undamped natural period, the damped natural circular frequency and the damped natural period, respectively. Furthermore the damping ratio, the displacement of the mass relative to the ground (deformation of the system) and the restoring force in the model are denoted by h , u , and f , respectively. The time derivative is designated by an over-dot.

EXTRACTION OF MAXIMUM DEFORMATION OF BASE-ISOLATION STORY FROM THE TOTAL RESPONSE

Once the maximum deformation of the equivalent SDOF model is obtained, the corresponding maximum deformation of the base-isolation story can be derived. This procedure is explained in this section.

The total displacement u consists of several components including the base mat displacement and rotation (see **Figure 8**).

$$u = u_U + u_I + u_H + \theta_R H \tag{20}$$

Then the corresponding deformation of the base-isolation story can be expressed by

$$u_I = u - (u_U + u_H + \theta_R H) = u - f_R \left(\frac{1}{k_U} + \frac{1}{k_H} + \frac{H^2}{k_R} \right) \tag{21}$$

where f_R denotes the common restoring force in the respective spring. When the maximum displacement $u_{\max i}$ and the maximum restoring force $f_{R \max i}$ are substituted into u and f_R in Equation (21), the maximum deformation of the base-isolation story can be obtained. Here the maximum displacement after the first impulse is denoted by $u_{\max 1}$ and that after the second impulse by $u_{\max 2}$. The maximum restoring force in the base-isolation story can be obtained as follows depending on the input level.

[Case 1: $u_{I \max 1}$, $u_{I \max 2}$, Case 2: $u_{I \max 1}$]

$$f_{R \max i} = k_I u_{I \max i}, u_{I \max i} = \frac{k}{k_I} u_{\max i} \tag{22}$$

[Case 2: $u_{I \max 2}$, Case 3: $u_{I \max 1}$]

$$f_{R \max i} = f_y + \alpha k(u_{\max i} - d_y), u_{I \max i} = d_{yI} + \frac{\alpha k(u_{\max i} - d_y)}{\alpha_I k_I} \tag{23}$$

[Case 3: $u_{I \max 2}$]

$$f_{R \max 2} = \alpha k(u_{p2} - u_{p1}), u_{I \max 2} = d_{yI} + \frac{\alpha k(u_{\max 2} - d_y)}{\alpha_I k_I} \tag{24}$$

Figure 9 shows the four cases depending on the input level. Case 3-1 is the case where the second impulse acts at the zero restoring force in the unloading process and Case 3-2 is the case where the second impulse acts at the zero restoring force in the reloading process.

ACCURACY INVESTIGATION OF THE PROPOSED TRANSFORMATION INTO SDOF SYSTEM THROUGH THE COMPARISON OF THE CLOSED-FORM EXPRESSION WITH THE RESULT BY TIME-HISTORY RESPONSE ANALYSIS

In order to investigate the reliability and validity of the proposed expression under the double impulse with the velocity V , the time-history response analyses of the 2DOF system on the swaying-rocking spring-dashpot system under the double impulse and the one-cycle sine wave have been performed. The second impulse was input at the timing such that the maximum deformation at the base-isolation story after the second impulse attains the maximum.

In this investigation, the adjustment of the input levels between the double impulse and the one-cycle sinusoidal wave is important from the viewpoint of the equivalence of the maximum Fourier amplitude. The adjustment procedure is explained in the references (Kojima and Takewaki, 2016; Kojima et al., 2017). The one-cycle sine wave corresponding to the critical double impulse can be expressed as follows.

$$\ddot{u}_g^{SW}(t) = 0.5\omega_p V_p \sin(\omega_p t) \quad (0 \leq t \leq T_p = 2t_0^c), \tag{25}$$

where $V_p/V = 1.2222$ (V : the velocity amplitude of the double impulse) (Kojima and Takewaki, 2016; Kojima et al., 2017). V_p denotes the maximum velocity of the sine wave. In addition, $T_p = 2t_0^c$ and $\omega_p = 2\pi/T_p$ denote the period and the circular frequency, respectively, of the sine wave. The critical time interval obtained above is used for t_0^c in Equation (25).

The swaying-rocking spring-dashpot parameters are evaluated by the formula due to Parmelee (1970) and are expressed by

$$k_H = \frac{6.77}{1.79 - \nu} Gr, k_R = \frac{2.52}{1.00 - \nu} Gr^3, c_H = \frac{6.21}{2.54 - \nu} \rho V_s r^2, c_R = \frac{0.136}{1.13 - \nu} \rho V_s r^4 \tag{26}$$

where the Poisson's ratio $\nu = 0.35$, the mass density $\rho = 1.8[t/m^3]$, the shear wave velocity $V_s = 200, 133, 100$ [m/s] for Soil type 1, 2, 3 (the shear modulus $G = \rho V_s^2$). The radius of the disk equivalent to the superstructure floor and the foundation mat is given by $r = \sqrt{m_U/(N\pi)}$ (assuming 1.0×10^3 (kg/m²) for the superstructure floor mass and 2.0×10^3 (kg/m²) for the

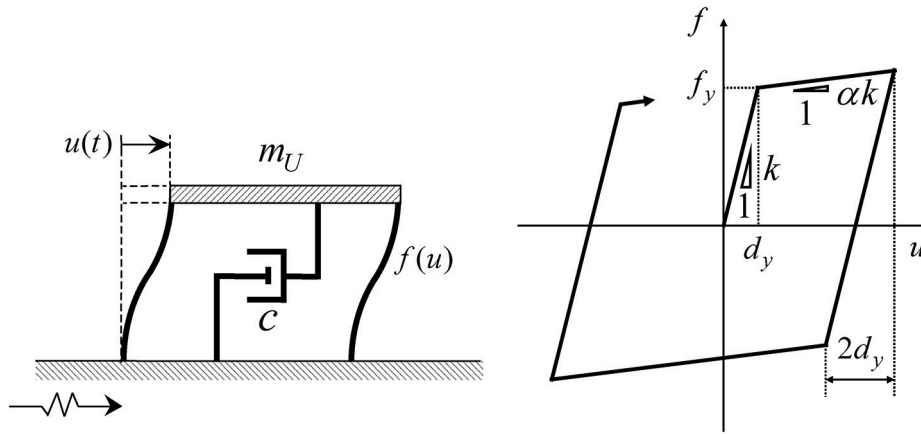


FIGURE 7 | Damped SDOF model and its bilinear hysteretic restoring-force characteristic.

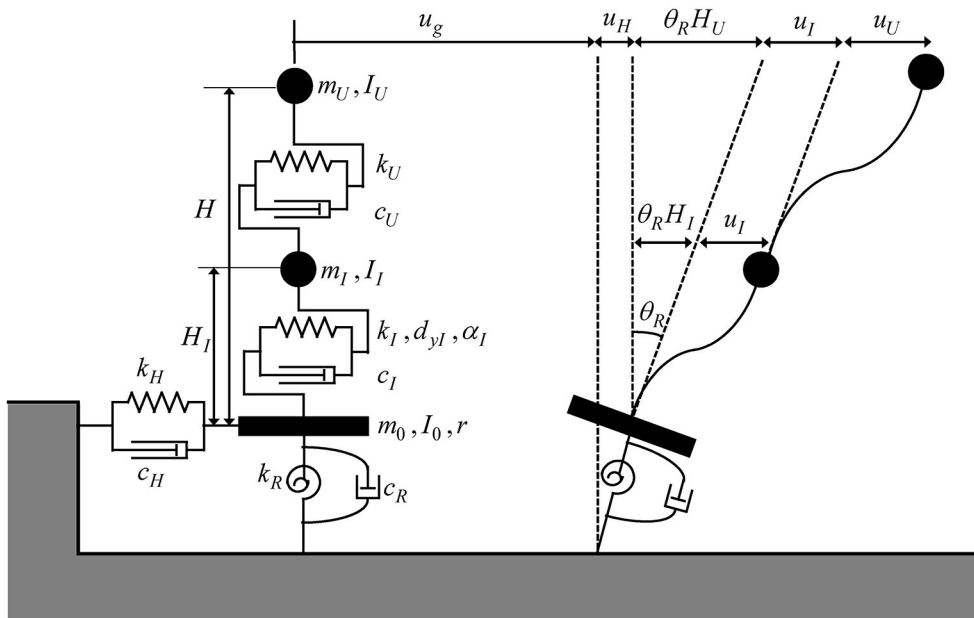


FIGURE 8 | Relation of deformation components.

foundation mat). The mass moments of inertia are given by $I_U = \pi r^2 \times m_U / 12$, $I_I = I_0 = \pi r^2 \times m_I / 12$. Let T_s and T_{BI} denote the fundamental natural period of the super structure with fixed base and the fundamental natural period of the rigid super structure on the base-isolation story.

The given parameters for the **10-story model** are as follows:

$$m_U = 800 \text{ [t]}, m_I = m_0 = 160 \text{ [t]}$$

$$T_s = 1.0 \text{ [s]}, k_U = (2\pi/T_s)^2 \times m_U = 3.16 \times 10^7 \text{ [N/m]}$$

$$h_U = 0.02, c_U = 2 \times \sqrt{m_U k_U} \times h_U = 2.01 \times 10^5 \text{ [(N \cdot s)/m]}$$

$$T_{BI} = 2.0 \text{ [s]}, k_I = (2\pi/T_{BI})^2 \times (m_U + m_I) = 9.47 \times 10^6 \text{ [N/m]}, \alpha_I = 0.1, d_{yI} = 0.01 \text{ [m]}$$

$$h_I = 0.02, c_I = 2 \times \sqrt{(m_U + m_I) k_I} \times h_I = 1.21 \times 10^5 \text{ [(N \cdot s)/m]}$$

On the other hand, the given parameters for the **20-story model** are as follows:

$$m_U = 1600 \text{ [t]}, m_I = m_0 = 160 \text{ [t]}$$

$$T_s = 2.0 \text{ [s]}, k_U = (2\pi/T_s)^2 \times m_U = 1.58 \times 10^7 \text{ [N/m]}$$

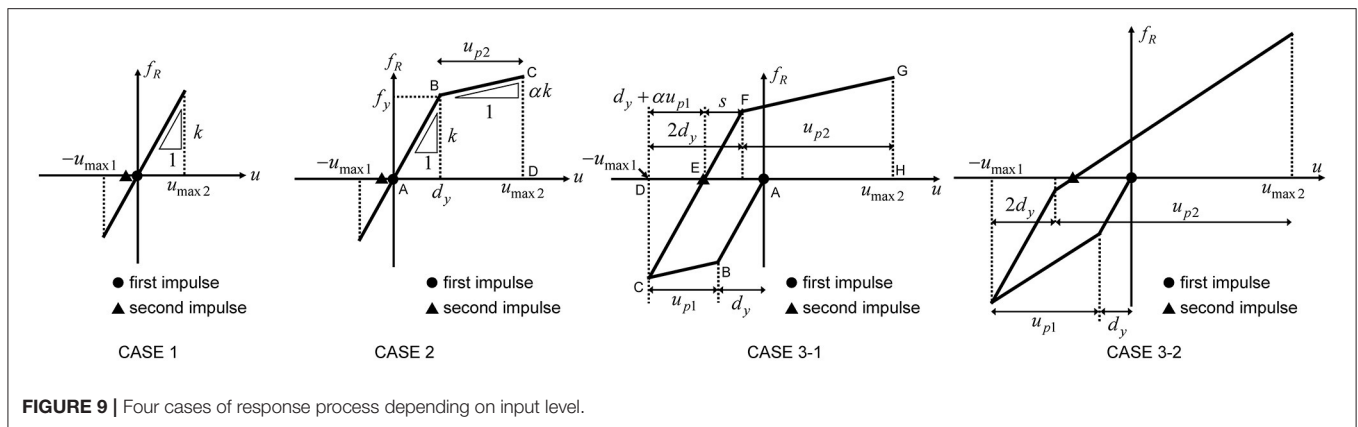


FIGURE 9 | Four cases of response process depending on input level.

$$h_U = 0.02, c_U = 2 \times \sqrt{m_U k_U} \times h_U = 2.01 \times 10^5 \text{ [(N} \cdot \text{s)/m]}$$

$$T_{BI} = 1.4 \text{ [s]}, k_I = (2\pi/T_{BI})^2 \times (m_U + m_I) = 3.55 \times 10^7 \text{ [N/m]}, \alpha_I = 0.1, d_{yI} = 0.01 \text{ [m]}$$

$$h_I = 0.02, c_I = 2 \times \sqrt{(m_U + m_I)k_I} \times h_I = 3.16 \times 10^5 \text{ [(N} \cdot \text{s)/m]}$$

The left figure in **Figure 10A** presents the comparison of the maximum deformations (after the first impulse and after the second impulse) of the models reduced from a 10-story building on the rigid ground. The solid line indicates the values based on the explicit expression explained in section Explicit Expression on Critical Elastic-Plastic Response Under Double Impulse (Akehashi et al., 2018) and the broken line presents the maximum value (top displacement) by the time-history response analysis for the 2DOF system on the swaying-rocking spring-dashpot system under the critical double impulse. On the other hand, the dotted line illustrates the maximum value (top displacement) by the time-history response analysis for the 2DOF system on the swaying-rocking spring-dashpot system under the corresponding one-cycle sine wave. It can be observed that the closed-form expressions are accurate enough. Furthermore, the right figure in **Figure 10A** presents the comparison of the maximum deformations (after the first impulse and after the second impulse) in the base-isolation story of the model on the rigid ground. The solid line indicates the value extracted from the explicit expression (Akehashi et al., 2018) as explained in section Extraction of Maximum Deformation of Base-Isolation Story from the Total Response and the broken line presents the maximum value in the base-isolation story by the time-history response analysis for the 2DOF system on the swaying-rocking spring-dashpot system under the critical double impulse. On the other hand, the dotted line illustrates the maximum value in the base-isolation story by the time-history response analysis for the 2DOF system on the swaying-rocking spring-dashpot system under the corresponding one-cycle sine wave. It can also be observed that the extracted values based on the closed-form expressions are accurate enough.

The left figure in **Figure 10B** shows the comparison of the maximum deformations (after the first impulse and after

the second impulse) of the models on the ground of type 1. Furthermore, the right figure in **Figure 10B** presents the comparison of the maximum deformations (after the first impulse and after the second impulse) in the base-isolation story of the model on the ground of type 1. It can be observed that the proposed model reduction into the SDOF model and the closed-form expressions in the previous paper (Akehashi et al., 2018) are sufficiently accurate in the evaluation of both the total response and the response in the base-isolation story.

Figures 10C,D illustrate the corresponding comparison for the models on the ground of type 2, 3. It can be seen that the proposed model reduction into the SDOF model and the closed-form expressions in the previous paper (Akehashi et al., 2018) are accurate enough for the models on rather soft grounds.

Figures 11A–D show the comparisons for the model reduced from a 20-story building on the rigid ground and the grounds of type 1–3. It can be seen that the same tendency as in the model of 10 story exists.

APPLICABILITY OF PROPOSED THEORY TO ACTUAL NEAR-FAULT GROUND MOTION

It seems important to investigate the applicability of the present theory to actual recorded pulse-type ground motions.

Consider a representative pulse-type ground motion, i.e., the Rinaldi station fault-normal component, shown in **Figure 1A**, during the Northridge earthquake in 1994. Since the ground motion is fixed, the structural models are varied, i.e., ω_1 and d_y in $V_y = \omega_1 d_y$ are varied. **Figure 12** shows the maximum amplitude of deformation for the recorded ground motion and the corresponding proposed one. The sum $(u_{\max 1} + u_{\max 2})/d_y$ of the both amplitudes is plotted here. The 10-story model on Soil types 1, 3 and the 20-story model on Soil type 1 and 3 introduced in the previous section are dealt with. The second stiffness ratios α of the equivalent SDOF model are 0.146, 0.202, 0.423, 0.649 for the 10-story model on Soil type 1 and 3 and the 20-story model on Soil types 1, 3, respectively. The damping ratios of the equivalent SDOF model are 0.0135, 0.0088, 0.0058, 0.0024 for the 10-story

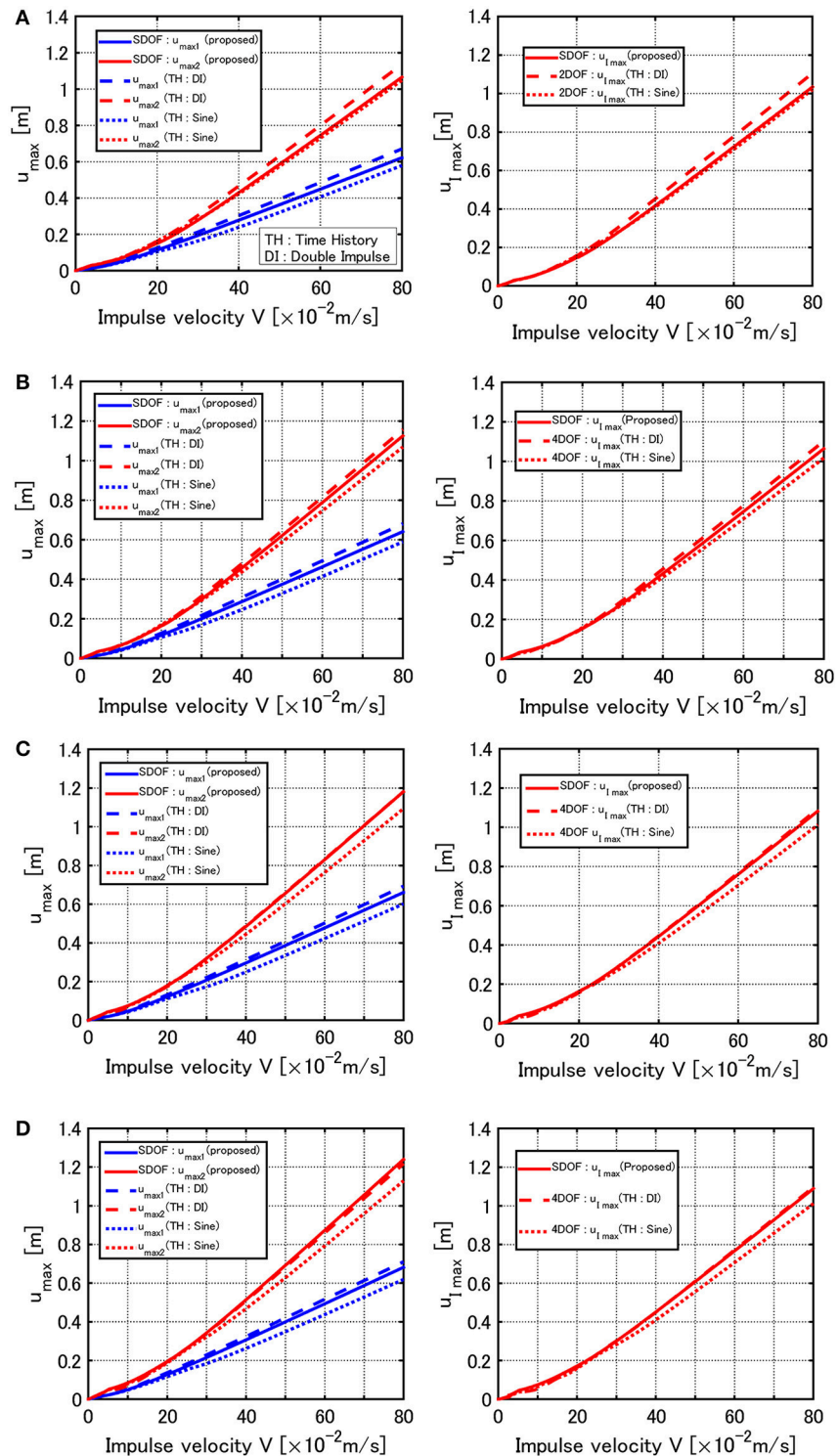


FIGURE 10 | Comparison of maximum deformations of 10-story model for the proposed method, time-history response analysis under double impulse and time-history response analysis under corresponding one-cycle sine wave, (A) Rigid ground, (B) Soil type 1, (C) Soil type 2, (D) Soil type 3.

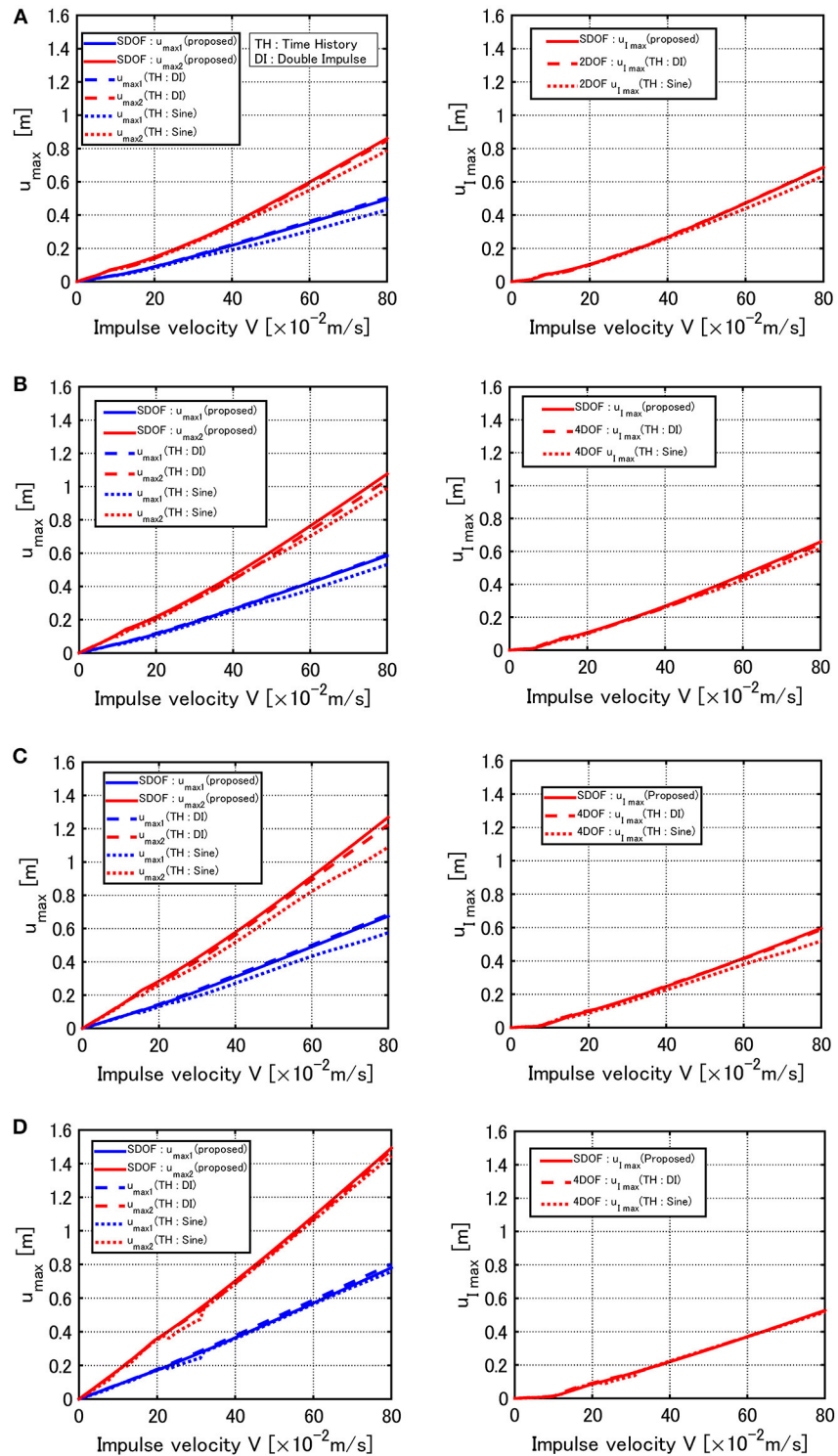
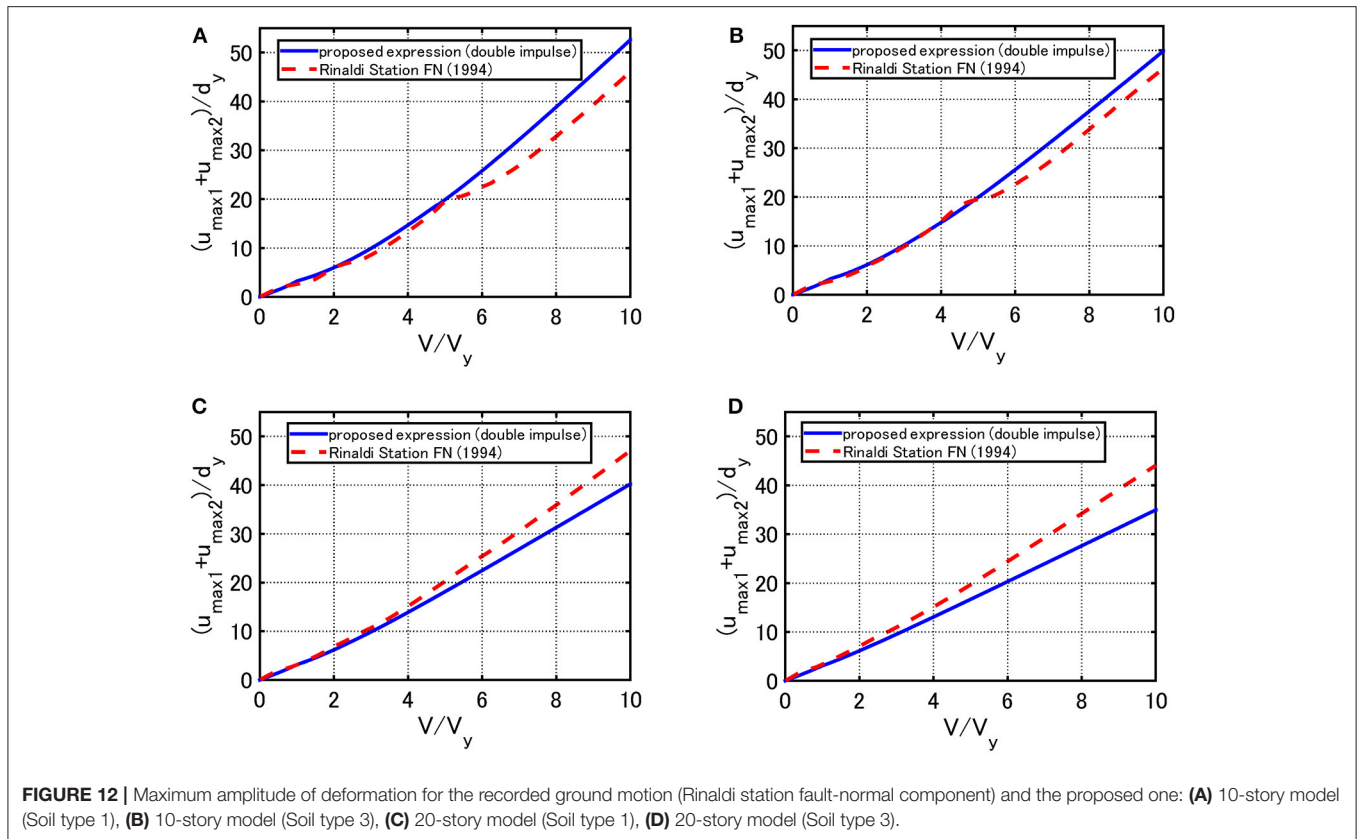


FIGURE 11 | Comparison of maximum deformations of 20-story model for the proposed method, time-history response analysis under double impulse and time-history response analysis under corresponding one-cycle sine wave, **(A)** Rigid ground, **(B)** Soil type 1, **(C)** Soil type 2, **(D)** Soil type 3.



model on Soil types 1, 3 and the 20-story model on Soil types 1, 3, respectively. As stated before, since the initial velocity V is determined in **Figure 1A**, V_y is changed here. This procedure is similar to the well-known elastic-plastic response spectrum developed in 1960-1970. The solid line is obtained by changing V_y for the specified V using the method for the double impulse and the dotted line is drawn by conducting the nonlinear time-history response analysis on each model with varied V_y under the recorded ground motion. It can be observed that, although a slight difference can be seen in the large range of V/V_y , the result by the proposed method is a fairly good approximate of the result by the recorded pulse-type ground motion.

CONCLUSIONS

The critical nonlinear response has been investigated for a base-isolated building on ground under a double impulse as a substitute for near-fault earthquake ground motions. The complicated model of the nonlinear base-isolated building considering soil-structure interaction was first modeled as a two-degree-of-freedom (2DOF) system (SDOF superstructure and base-isolation story) on a swaying-rocking spring-dashpot system. Then the 2DOF system on a swaying-rocking spring-dashpot system was transformed into an SDOF system on a swaying-rocking spring-dashpot system by neglecting the mass on the base-isolation story. Finally the SDOF system on a swaying-rocking spring-dashpot system was further transformed

into an SDOF system by neglecting the mass and moment of inertia of the base mat. Since an explicit expression had been derived on the maximum elastic-plastic response of an SDOF damped structure with bilinear hysteresis under the “critical double impulse input,” this expression was applied to the finally transformed SDOF system. The transformation of structural viscous damping in different elements is another new aspect in this paper. The conclusions may be summarized as follows.

- (1) By deleting the degrees of freedom just above the base-isolation story and at the base mat, the complicated model of the nonlinear base-isolated building considering soil-structure interaction can be transformed into an SDOF system. After the model transformation, the explicit expression on the critical maximum elastic-plastic response of the SDOF system derived in the previous paper can be used and the critical maximum elastic-plastic response in the base-isolation story can also be evaluated.
- (2) The accuracy of the model transformation and the validity in the application of the previously derived expressions have been discussed. The comparison with the maximum deformation under the critical double impulse and the one-cycle sine wave (good representative of the main part of the near-fault ground motion) supports the validity of the proposed theory. For reliable comparison, the time-history analysis was introduced. It has been made clear that, once

appropriate adjustment of the maximum Fourier amplitude is made, the double impulse can become a reasonable substitute for the one-cycle sine wave. Furthermore the maximum deformation subjected to a near-fault ground motion can be captured by the double impulse.

- (3) Numerical examples demonstrated that the explicit expression on the critical maximum elastic-plastic response of the SDOF system derived in the previous paper can simulate the maximum response under the critical double impulse and the corresponding one-cycle sine wave within a good accuracy independent of the ground stiffness. While the proposed expression almost gives an upper bound of the time-history response under the double impulse and the corresponding one-cycle sine wave in 20-story models, it does not necessarily provide an upper bound of the response under the double impulse in 10-story models. However its discrepancy is small. In addition, the influence of the ground stiffness on the critical maximum deformation at the base-isolation story is small.
- (4) The expression on the critical maximum elastic-plastic response of the SDOF system under the double impulse

can simulate the maximum response under recorded ground motions once the main part of the recorded ground motion is transformed into the double impulse. In this simulation, the yield deformation and the natural circular frequency are changed because the recorded ground motion is fixed.

AUTHOR CONTRIBUTIONS

HA formulated the problem, conducted the computation, and wrote the paper. KK formulated the problem and wrote the paper. KF helped the computation and wrote the paper. IT supervised the research and wrote the paper.

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Conflict of Interest Statement: The authors declare that the research was conducted in the absence of any commercial or financial relationships that could be construed as a potential conflict of interest.

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